ARDROSSAN FARM-WEST PARCEL TRANSPORTATION IMPACT STUDY

July 26, 2013 TPD # ESII.A.00001

Radnor Township Delaware County, PA





For Submission To:

Radnor Township

Prepared By:



TRAFFIC PLANNING AND DESIGN, INC.

www.TrafficPD.com

TPD Services: Roadway Design Bridge Design & Inspection Transportation Planning Traffic Signal System Design Municipal Services Environmental Services Construction Management & Inspection

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For Submission to:

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Prepared For:

ESIII LP Chadds Ford, PA 19317

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EXECUTIVE SUMMARY

The purpose of this study is to examine the potential traffic impact associated with the proposed Ardrossan Farm (west parcel) development on the roadway network in Radnor Township, Delaware County, PA. Based on this evaluation, the following conclusions were reached:

- The project scope and the extent of the study area were confirmed with representatives of Township staff.
- The project site is located on the western side of Darby-Paoli Road (S.R. 1015), between Saw Mill Road and Godfrey Road (S.R. 1036). The proposed development will consist of 6 single-family homes.
- The site will be served by one driveway to Darby-Paoli Road (S.R. 1015), between Saw Mill Road and Godfrey Road. Please note, Darby-Paoli Road is a state-maintained roadway and thus will require a PennDOT HOP. Therefore, the proposed development is subject to PennDOT's review and approval.
- The measured sight distances at the proposed site driveway exceed PennDOT's sight distance requirements.
- The proposed development is expected to generate 5 new vehicle-trips during the weekday A.M. peak hour and 6 new vehicle-trips during the weekday P.M. peak hour.
- Under 2020 projected conditions with the development of the proposed site, the study area intersections will operate at the same overall intersection level of service (ILOS) as under 2020 base conditions, thus complying with the requirement outlined in PennDOT's TIS Guidelines.
- Under 2020 projected conditions, all levels of service at the site driveway will comply with the requirement outlined in PennDOT's TIS Guidelines.
- Traffic Planning and Design Inc. (TPD) recommends the following roadway improvements as outlined at the study area intersections:
 - Complete spot check counts when school is in session to verify the volumes utilized in this study.
 - o Coordinate mutually agreed upon potential safety enhancement improvements with the Township along the roadways in the vicinity of the site.
 - Design the proposed site driveway to Darby-Paoli Road in accordance to the applicable PennDOT standards.



• Levels of Service (LOS) for the study area intersections have been summarized in matrix form. **Table I** details the overall intersection LOS for each study area intersection.

TABLE I INTERSECTION LEVELS OF SERVICE (DELAY) SUMMARY

	Weekday A.M. Peak Hour			Weekday P.M. Peak Hour		
Intersection	2013	2020 Build-Out Year		2013	2020 Build	d-Out Year
	Existing	Base	Projected	Existing	Base	Projected
Darby Paoli Road & Saw Mill Road	B (12.9)	C (16.3)	C (16.7)	A (4.4)	A (5.2)	A (5.2)
Darby Paoli Road & Godfrey Road	A (2.3)	A (2.4)	A (2.4)	A (3.6)	A (3.8)	A (3.9)
Darby Paoli Road & Site Driveway			A (0.1)			A (0.0)

Base = No-Build scenario; Projected = Build scenario; ILOS = Overall Intersection Level of Service



INTRODUCTION

Traffic Planning and Design, Inc. (TPD) has completed a Transportation Impact Study (TIS) for the proposed Ardrossan Farm (west parcel) development in Radnor Township, Delaware County, Pennsylvania. The project site is located on the western side of Darby-Paoli Road (S.R. 1015), between Saw Mill Road and Godfrey Road (S.R. 1036), as shown in **Figure 1**. As shown in **Figure 2**, the proposed development will consist of 6 single-family homes.

The project scope and the extent of the study area were confirmed with representatives of Township staff.

Site Access Location

The site will be served by one driveway to Darby-Paoli Road (S.R. 1015), between Saw Mill Road and Godfrey Road. Please note, Darby-Paoli Road is a state-maintained roadway and thus will require a PennDOT HOP. Therefore, the proposed development is subject to PennDOT's review and approval.

EXISTING ROADWAY NETWORK

A field review of the existing roadway system in the study area was conducted. The existing roadway characteristics within the study area are summarized in **Table 1**. The existing lane configuration and intersection controls for the study area intersections are shown in **Figure 3**. Photographs of the study area intersections are included in **Appendix A**.

TABLE 1
ROADWAY CHARACTERISTICS WITHIN STUDY AREA

Roadway	Ownership	PennDOT Functional Classification/ Roadway Type	Predominant Directional Orientation	Average Daily Traffic ¹	Posted Speed Limit
Darby-Paoli Road	State (S.R. 1015)	Urban Collector/ Urban Minor Arterial	North-South	6,820	35 mph
Saw Mill Road	Township	Urban Collector	East-West	1,674	25 mph
Godfrey Road	State (S.R. 1036)	Local Road	East-West	1,175	25 mph

1. Based on PennDOT iTMS data.

Crash Data Investigation

Crash data were obtained from PennDOT for the study area intersections. PennDOT defines a <u>reportable</u> crash as follows, "A <u>reportable</u> (crash) is one in which an injury or fatality occurs or if at least one of the vehicles involved requires towing from the scene." <u>Reportable</u> crashes were



tabulated for the five-year time period beginning 1/1/2008 and ending 12/31/2012. The number of reportable crashes at the study area intersections is shown in **Table 2**.

TABLE 2
PENNDOT <u>REPORTABLE</u> CRASH DATA

	Number of Reportable Crashes					
Study Area Intersection	2008	2009	2010	2011	2012	
Darby-Paoli Road and Saw Mill Road	3	0	1	0	1	
Darby-Paoli Road and Godfrey Road	0	0	1	0	0	

EXISTING TRAFFIC CONDITIONS

Manual Turning Movement Counts

Manual traffic counts were conducted on 15-minute intervals during the weekday morning (7:00 to 9:00 A.M.) and weekday evening (4:00 to 6:00 P.M.) peak periods. Peak hours and count dates for the study area intersections are identified in **Table 3**.

TABLE 3
MANUAL TRAFFIC COUNT INFORMATION

Intersection	Date of Traffic Counts	Time Period	Intersection Peak Hour ¹
Darby-Paoli Road and	Wednesday, July 17, 2013	Weekday A.M.	8:00 to 9:00 A.M.
Saw Mill Road	Wednesday, July 17, 2013	Weekday P.M.	5:00 to 6:00 P.M.
Darby-Paoli Road and	Wednesday, July 17, 2013	Weekday A.M.	8:00 to 9:00 A.M.
Godfrey Road	Wednesday, July 17, 2013	Weekday P.M.	5:00 to 6:00 P.M.

^{1.} Peak Hour consists of the four consecutive 15-minute intervals where the highest traffic volumes occur.

Existing condition traffic volumes for the weekday A.M. and weekday P.M. peak hours are illustrated in **Figure 4**. Manual traffic count data sheets are provided in **Appendix B**.

Automatic Traffic Recorder Counts

Automatic Traffic Recorder (ATR) counts were conducted along the following roadways in the vicinity of the proposed site:

- Darby-Paoli Road, between Newtown Road and Saw Mill Road;
- Newtown Road, between Darby-Paoli Road and Abrahams Lane.



The ATR counts were conducted from Monday, July 15, 2013 until Friday, July 19, 2013. The ATR data sheets are provided in **Appendix B**.

Seasonal Count Comparison

In order to evaluate seasonal fluctuations in traffic TPD compared the Average Daily Traffic (ADT) volumes on Newtown Road and Darby-Paoli Road based on the ATR data to the PennDOT iTMS. In addition, TPD compared the peak volumes from the manual traffic counts for these same roadways to the peak hour volumes as determined based on PennDOT's iTMS data (i.e. K-factor). Based on this comparison, the existing traffic volumes collected are comparable to the PennDOT traffic data. Therefore, no seasonal adjustment was applied to the existing traffic volumes. However, please note, spot check counts will be completed when school is in session to verify the volumes utilized.

BASE (NO-BUILD) CONDITIONS

Annual Background Growth

A background growth factor for the roadways in the study area was developed based on growth factors for September 2012 to July 2013 obtained from the PennDOT Bureau of Planning and Research (BPR). The PennDOT BPR suggests using a background growth trend factor of 0.51% per year in Delaware County for urban non-interstate roadways. As such, the background growth factor was applied annually to yield an overall growth percentage of 3.6% (0.51% per year, compounded over 7 years) for the 2020 build-out.

Nearby Proposed Developments

Base (no-build) traffic conditions were calculated to include traffic volumes from proposed developments, which, though not operating under existing conditions may be operating by 2020. The following nearby planned development was specifically included in this study:

Ardrossan Farm (East Parcel) is a proposed residential development consisting of a maximum of 87 single-family homes. The site is located on the southeast corner of the Newtown Road (S.R. 1021)/Darby-Paoli Road (S.R. 1015) intersection. Access is proposed to Darby-Paoli Road and Newtown Road via multiple driveways. Trip distributions for this development were obtained from the July 2013 study prepared by TPD.

The additional traffic volumes due to background growth and the background development were added to the existing traffic data to produce 2020 base (no-build) condition traffic volumes. The 2020 base condition volumes for the weekday A.M. and weekday P.M. peak hours are illustrated in **Figure 5**. Volume development spreadsheets are included in **Appendix C**.



SCHEDULED ROADWAY IMPROVEMENTS

Programmed Improvements

Based on a review of the DVRPC Transportation Improvement Program (TIP) there are two (2) bridges on Darby Paoli Road over the Little Darby Creek and Wigwam Run which will either be rehabilitated or replaced.

PROPOSED SITE ACCESS

Sight Distance Analysis

A sight distance analysis was prepared for the proposed site driveway. In general, recommended safe sight distances depend upon the posted speed limit and roadway grades. The existing sight distances at the proposed driveways were measured in accordance with PennDOT Publication 282 Highway Occupancy Permit Guidelines and compared to PennDOT's desirable sight distance standard, which is identified in 67 PA Code Chapter 441.8(h), "Access to and Occupancy of Highways by Driveways and Local Roads." In addition, measured sight distances at the proposed driveways were compared to PennDOT's safe stopping sight distance standard, which is calculated by the following equation:

SSSD = $1.47VT + V^2/[30(f\pm g)]$

SSSD = safe stopping sight distance (acceptable sight distance)

V = Vehicle Speed

T = Perception Reaction Time of Driver (2.5 seconds)

f = Coefficient of Friction for Wet Pavements

g = Percent of Roadway Grade Divided by 100

Table 4 shows the measured, desirable, acceptable (SSSD), and required sight distances at the site driveways for vehicles entering and exiting the site.

TABLE 4
SIGHT DISTANCE ANALYSIS
SITE DRIVEWAY TO DARBY-PAOLI ROAD

		Posted	1	Sight Distances (feet)		
	Direction	Speed	Grade ¹	DES	SSSD	EXIST
Exiting	To the left	35 mph	+3%	440	239	500+
Movements	To the right	35 mph	-5%	350	269	500+
Entering	Approaching same direction	35 mph	-5%	300	269	400+
Left Turns	Approaching opposite direction	35 mph	+3%	300	239	400+

DES = PennDOT Desirable Sight Distance

SSSD = PennDOT Acceptable Sight Distance

EXIST = Existing (measured) Sight Distance

1 = Roadway Grade Approaching Driveway



As shown in **Table 4** above, the measured sight distances at the site driveways exceed PennDOT's sight distance requirements.

TRIP GENERATION

The trip generation rates for the proposed development were obtained from the manual <u>Trip Generation</u>, Ninth Edition, 2012, an Institute of Transportation Engineers (ITE) Informational Report. The statistics in <u>Trip Generation</u> are empirical data based on more than 4,800 trip generation studies. The data are categorized by Land Use Codes, with total vehicular trips for a given land use estimated using an independent variable and statistically generated rates or equations.

For the proposed development, Land Use Codes 210 (Single-Family Detached Housing) was used to calculate the number of vehicular trips the development will generate during the following time periods: (1) average weekday; (2) weekday A.M. peak hour; and (3) weekday P.M. peak hour. **Table 5** shows the data for the analyzed time periods.

TABLE 5
ITE TRIP GENERATION DATA

Land Use	ITE#	Time Period	Equations/Rates	Entering %
Single-		Weekday	T = 9.52*(X)	50%
Family	210	Weekday A.M. Peak Hour	T = 0.75*(X)	25%
Homes		Weekday P.M. Peak Hour	T = 1.00*(X)	63%

T = number of site-generated vehicular trips X = independent variable (units)

Table 6 summarizes the site trip generation for the analyzed time periods.

TABLE 6 ARDROSSAN FARM (WEST PARCEL) TRIP GENERATION SUMMARY

Time Period	Enter	Exit	Total
Weekday (24-Hour Total)	29	29	58
Weekday A.M. Peak Hour		4	5
Weekday P.M. Peak Hour	4	2	6

As shown in **Table 6**, the proposed development is expected to generate 5 new vehicle-trips during the weekday A.M. peak hour and 6 new vehicle-trips during the weekday P.M. peak hour.

TRIP DISTRIBUTION

The distribution of trips generated by the proposed development was based on the local road network, the existing traffic patterns, the proposed use of the site, and the site driveway



locations. The new trips for the proposed development were distributed to the local roadway network based on the percentages shown in **Table 7**.

TABLE 7
TRIP DISTRIBUTION PERCENTAGES – NEW TRIPS

Direction - To/From	Assignment (To/From)	Distribution Percentage
East	via Godfrey Road	22%
West	via Saw Mill Road	15%
North	via Darby-Paoli Road	55%
South	via Darby-Paoli Road	8%

The assignment of site-generated trips for the proposed development during the weekday A.M. and P.M. peak hours are shown in **Figure 6**.

PROJECTED (BUILD) CONDITION TRAFFIC VOLUMES

The site-generated trips for the proposed development were added to the 2020 base (no-build) condition traffic volumes to develop 2020 projected (build) condition traffic volumes. The 2020 projected condition traffic volumes for the weekday A.M. and P.M. peak hours are shown in **Figure** 7.

LEVELS OF SERVICE FOR AN INTERSECTION

For analysis of intersections, level of service is defined in terms of delay, which is a measure of driver discomfort and frustration, fuel consumption, and lost travel time. LOS criteria is stated in terms of control delay per vehicle for a one-hour analysis period. Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. The criteria are shown in **Table 8**. Delay, as it relates to level of service, is a complex measure and is dependent upon a number of variables. For signalized intersections, these variables include the quality of vehicle progression, the cycle length, the green time ratio, and the volume/capacity ratio for the lane group in question. For unsignalized intersections, delay is related to the availability of gaps in the flow of traffic on the major street and the driver's discretion in selecting an appropriate gap for a particular movement from the minor street (straight across, left or right turn).



TABLE 8 LEVEL OF SERVICE CRITERIA UNSIGNALIZED AND SIGNALIZED INTERSECTIONS ¹

	Control Delay Per Vehicle (Seconds)				
Level of Service	Signalized	Unsignalized			
A	≤10	<u>≤</u> 10			
В	> 10 and ≤ 20	> 10 and ≤ 15			
С	$>$ 20 and \leq 35	$> 15 \text{ and } \le 25$			
D	$> 35 \text{ and } \le 55$	$> 25 \text{ and } \leq 35$			
E	$>$ 55 and \leq 80	$> 35 \text{ and } \le 50$			
F	> 80	> 50			

Obtained from Exhibits 18-4 and 19-1 of the Transportation Research Board's Highway Capacity Manual 2010

CAPACITY ANALYSIS METHODOLOGY

Capacity analyses were conducted for the weekday A.M. and weekday P.M. peak hours at the study area intersections. The study area intersections were analyzed according to the methodologies contained in the 2010 *Highway Capacity Manual* (HCM) using *Synchro 8* software, a Trafficware product. The following conditions were analyzed:

- Existing conditions;
- 2020 Base conditions (Build-out year without development);
- 2020 Projected conditions (Build-out year with development).

In addition, capacity analyses were conducted at the site driveway intersection under the 2020 projected conditions. The capacity analysis worksheets are included in **Appendix D**.

PennDOT's Transportation Impact Study Guidelines outlined in Strike-Off Letter 470-09-4, dated February 12, 2009 contain the following criteria regarding levels of service:

- Page 29 of the Guidelines state that if evaluation of the With Development Horizon Year Scenario to the Without Development Horizon Year Scenario indicates that the overall intersection level of service has dropped, the applicant will be required to mitigate the level of service if the increase in overall intersection delay is greater than 10-seconds. If the overall intersection delay increase is less than or equal to 10-seconds, mitigation of the intersection will not be required.
- Page 29 of the Guidelines state that for mitigation scenarios, applicants are expected to mitigate the overall intersection LOS to the original Without Development LOS; the 10second delay variance is not applied to mitigation scenarios. Applicants may be required to address available storage and queue lengths at critical movements or approaches even if the overall LOS requirements are met.



- Page 31 of the Guidelines state that if signalization is the preferred alternative for mitigation, overall intersection LOS C in rural areas and LOS D in urban areas is acceptable.
- Page 31 of the Guidelines states new signalized or unsignalized intersection established to serve as access to the development shall be designed to operate at minimum LOS C for rural areas, and minimum LOS D for urban areas.

LEVELS OF SERVICE IN THE STUDY AREA

Level of service (LOS) matrices for the study area intersections are shown in **Table 9** for the weekday A.M. and weekday P.M. peak hours. Per PennDOT standards, the signal timings at the signalized study area intersection have been optimized under base conditions and projected conditions.

TABLE 9 LEVEL OF SERVICE (DELAY) SUMMARY

		Weekday A.M. Peak Hour			Weekday P.M. Peak Hour		
Intersection	Movement	2013	2020 Build-Out Year 20	2013	2020 Buil	d-Out Year	
		Existing	Base	Projected	Existing	Base	Projected
D 1 D 12D 10	EB L/R	D	E (40.1)	E (40.9)	С	D	D
Darby-Paoli Road & Sawmill Road	NB L	Α	Α	A	Α	A	A
	ILOS	B (12.9)	C (16.3)	C (16.7)	A (4.4)	A (5.2)	A (5.2)
n 1 n 1:n 10	WB L/R	В	В	В	С	С	С
Darby-Paoli Road & Godfrey Road	SB L	Α	A	Α	Α	A	A
Godffey Koad	ILOS	A (2.3)	A (2.4)	A (2.4)	A (3.6)	A (3.8)	A (3.9)
Darby-Paoli Road &	EB L/R		, _	В			В
	NB L			A			A
Site Driveway	ILOS			A (0.1)			A (0.0)

Base = No-Build scenario; Projected = Build scenario; ILOS = Overall Intersection Level of Service

As shown in **Table 9**, under 2020 projected conditions with the development of the proposed site, the study area intersections will operate at the same overall intersection level of service (ILOS) as under 2020 base conditions, thus complying with the requirement outlined in PennDOT's TIS Guidelines.

Under 2020 projected conditions, all levels of service at the site driveway will comply with the requirement outlined in PennDOT's TIS Guidelines.



AUXILIARY TURN LANE ANALYSIS

TPD evaluated auxiliary turn lane warrants at the proposed site driveway. The warrant analysis was conducted according to the methodologies contained in Chapter 11 of PennDOT's *Publication 46* and Strike-Off Letter 470-08-07. **Table 10** summarizes the results of the auxiliary turn lane analysis.

TABLE 10 AUXILIARY TURN LANE ANALYSIS SUMMARY

Intersection	Auxiliary Lane	Warrant Satisfied?
Darby-Paoli Road and	NB Left-Turn Lane	No
Site Driveway	SB Right-Turn Lane	No

As shown in **Table 10**, the warrants are not satisfied for turn lanes on Darby-Paoli Road at the proposed site driveway. The calculations for the auxiliary turn lane warrants are included in **Appendix E**.

RECOMMENDATIONS

TPD has made the following recommendations in relation to the proposed Ardrossan Farm development in Radnor Township:

- Complete spot check counts when school is in session to verify the volumes utilized in this study.
- Coordinate mutually agreed upon potential safety enhancement improvements with the Township along the roadways in the vicinity of the site.
- Design the proposed site driveway to Darby-Paoli Road in accordance to the applicable PennDOT standards.

CONCLUSIONS

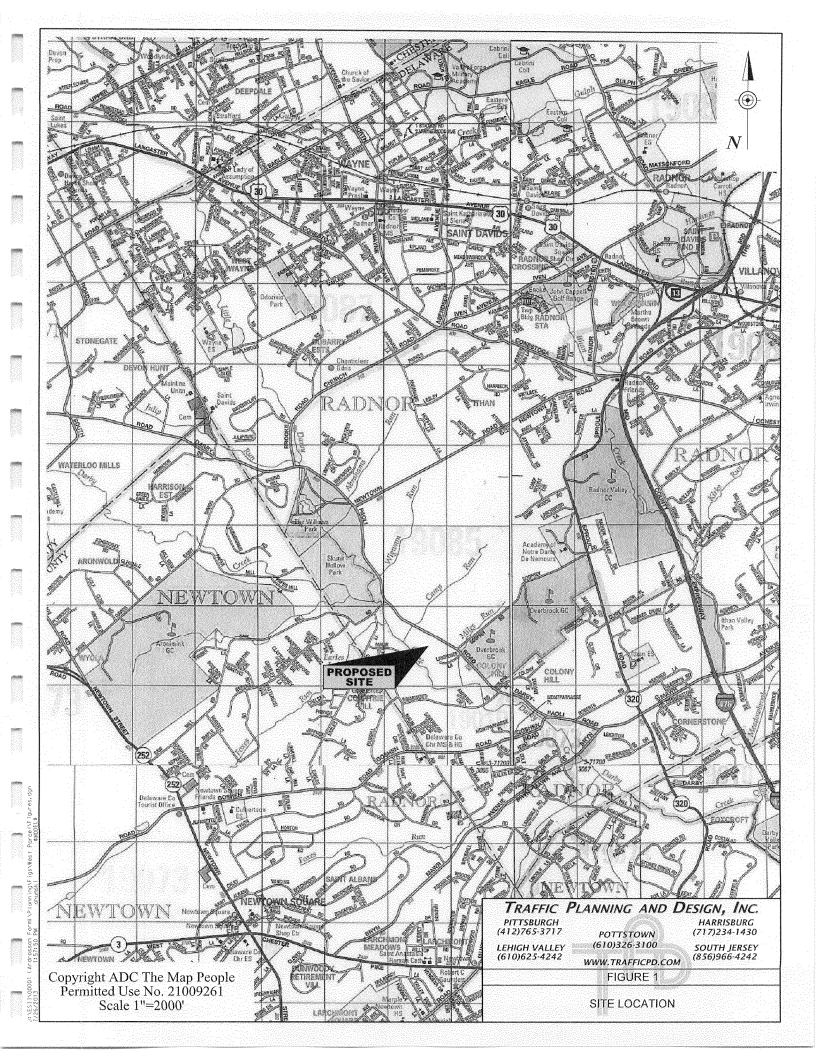
Based on the results of the transportation impact study, TPD offers the following conclusions:

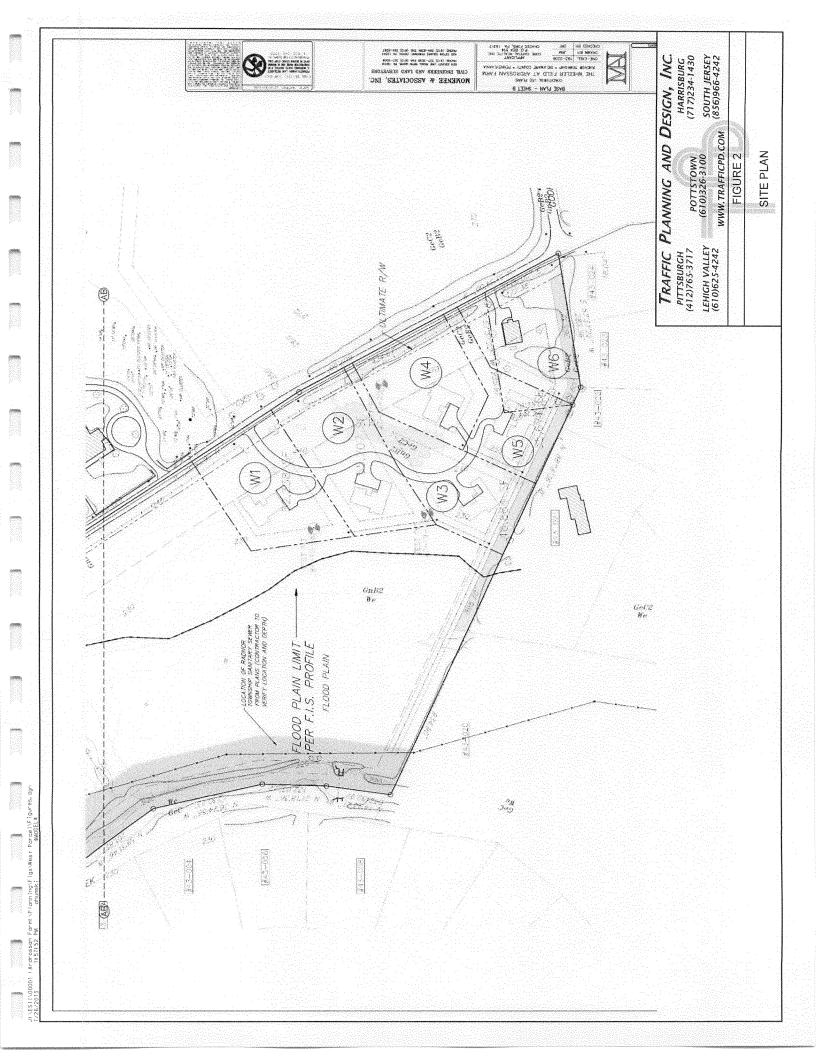
- The project scope and the extent of the study area were confirmed with representatives of Township staff.
- The project site is located on the western side of Darby-Paoli Road (S.R. 1015), between Saw Mill Road and Godfrey Road (S.R. 1036). The proposed development will consist of 6 single-family homes.
- The site will be served by one driveway to Darby-Paoli Road (S.R. 1015), between Saw Mill Road and Godfrey Road. Please note, Darby-Paoli Road is a state-maintained roadway and



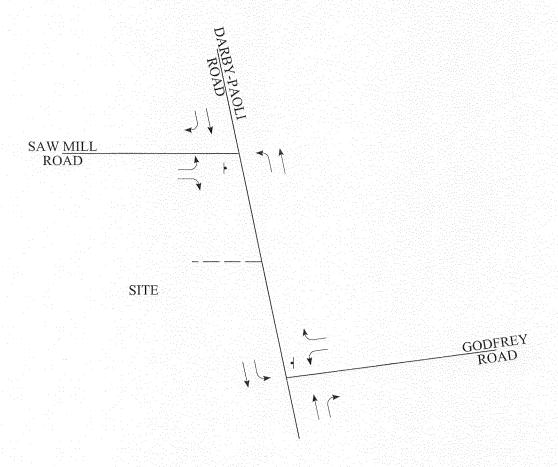
thus will require a PennDOT HOP. Therefore, the proposed development is subject to PennDOT's review and approval.

- The measured sight distances at the proposed site driveway exceed PennDOT's sight distance requirements.
- The proposed development is expected to generate 5 new vehicle-trips during the weekday A.M. peak hour and 6 new vehicle-trips during the weekday P.M. peak hour.
- Under 2020 projected conditions with the development of the proposed site, the study area intersections will operate at the same overall intersection level of service (ILOS) as under 2020 base conditions, thus complying with the requirement outlined in PennDOT's TIS Guidelines.
- Under 2020 projected conditions, all levels of service at the site driveway will comply with the requirement outlined in PennDOT's TIS Guidelines.









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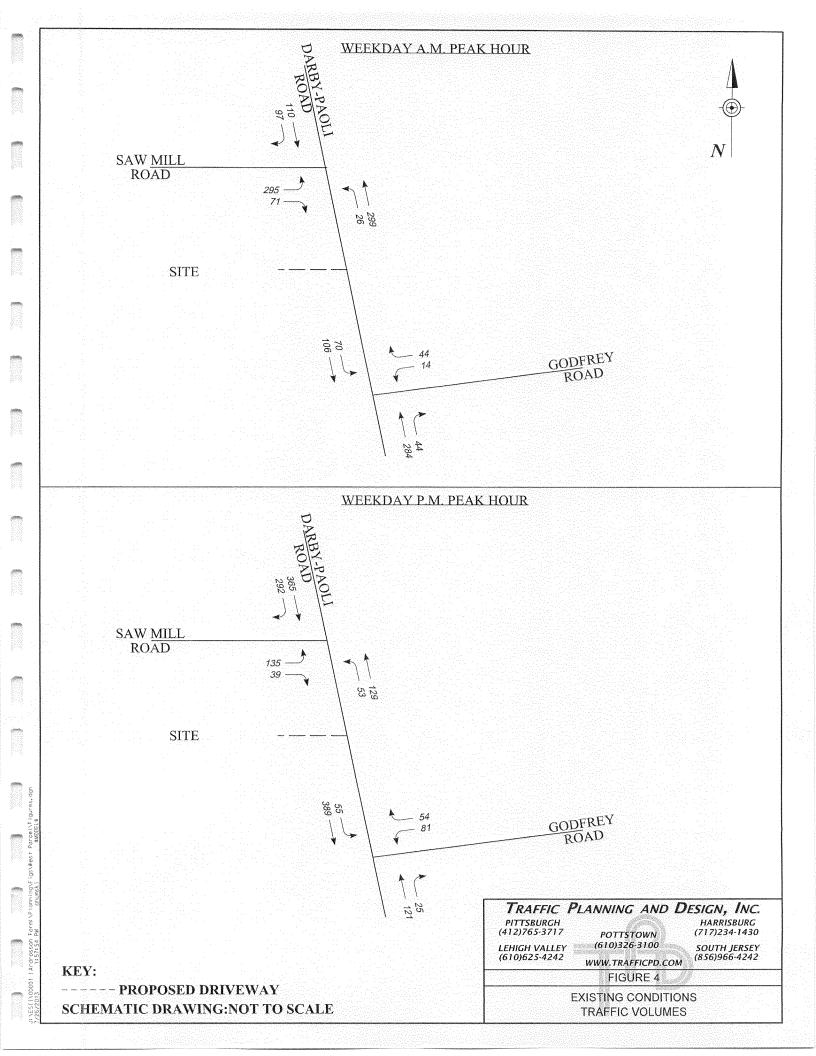
SOUTH JERSEY (856)966-4242

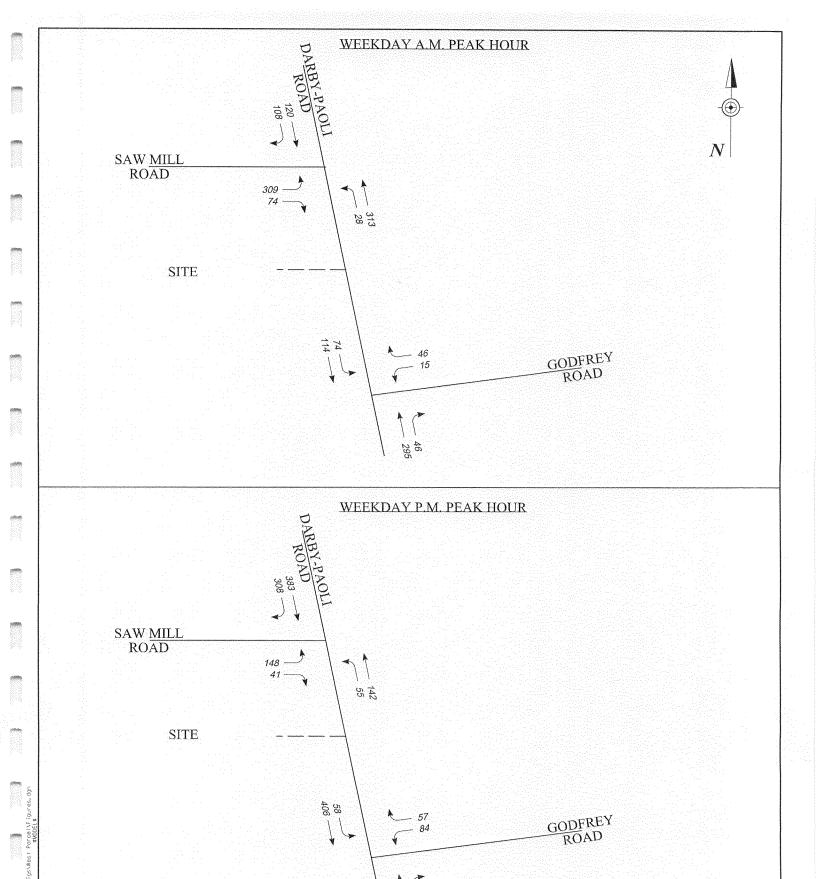
FIGURE 3

EXISTING LANE CONFIGURATIONS AND INTERSECTION CONTROLS

KEY:

---- PROPOSED DRIVEWAY SCHEMATIC DRAWING:NOT TO SCALE





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KEY:

--- PROPOSED DRIVEWAY SCHEMATIC DRAWING:NOT TO SCALE TRAFFIC PLANNING AND DESIGN, INC.

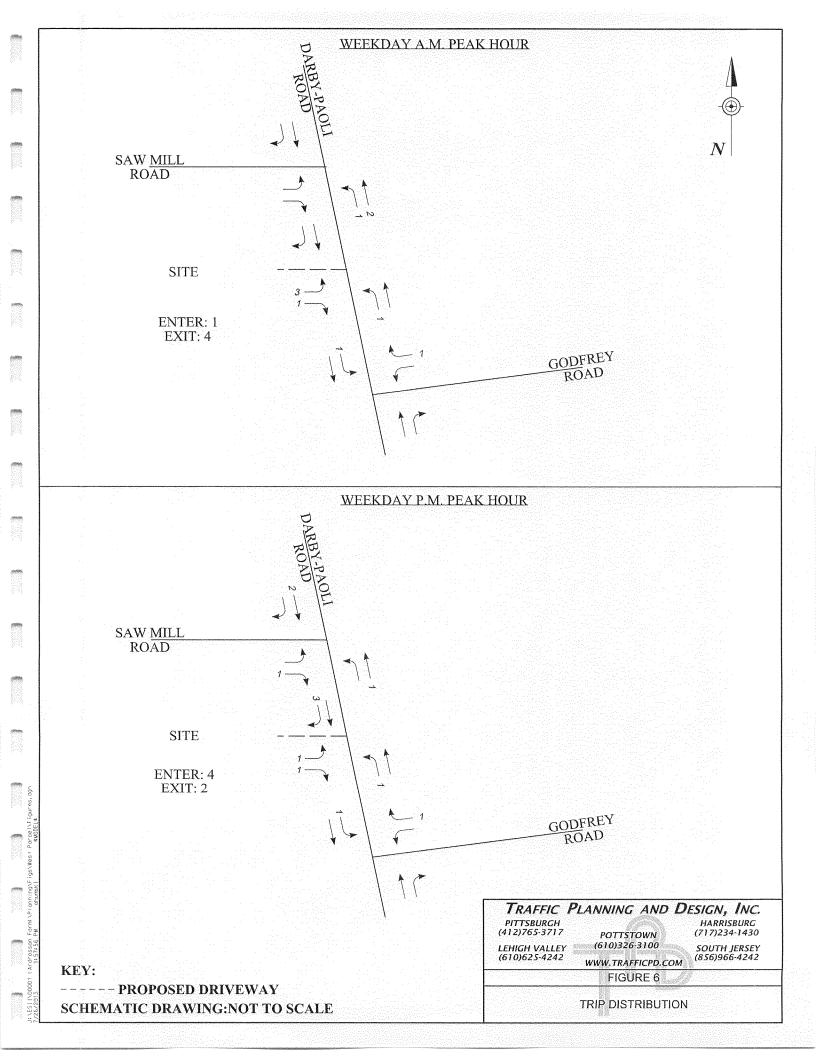
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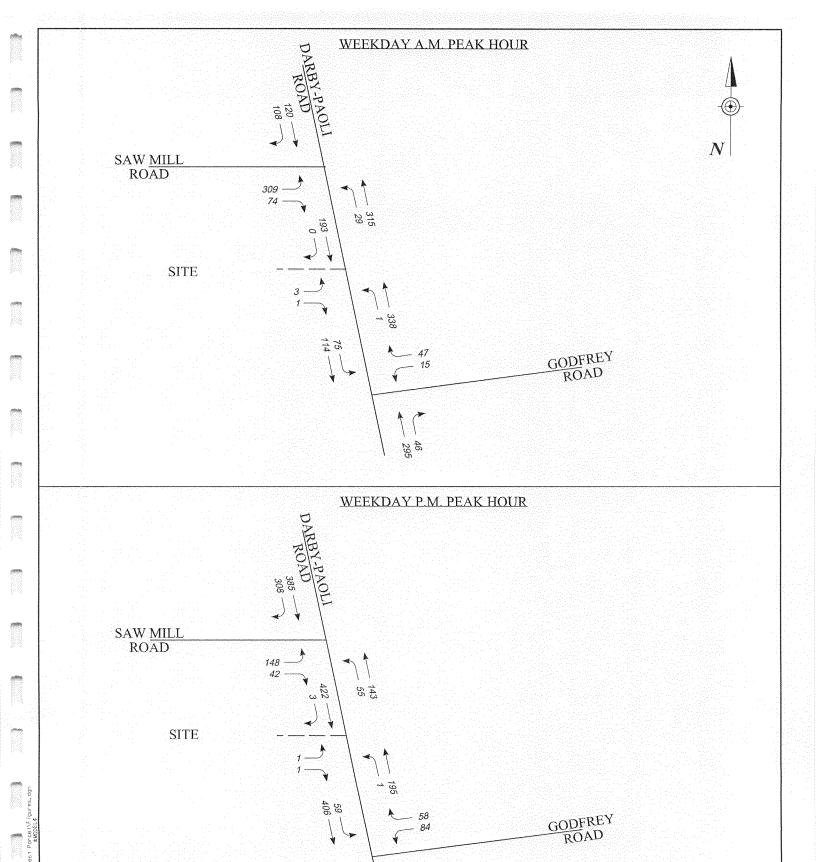
POTTSTOWN (610)326-3100 HARRISBURG (717)234-1430 SOUTH JERSEY (856)966-4242

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FIGURE 5

2020 BASE CONDITIONS TRAFFIC VOLUMES





. 26 130 TRAFFIC PLANNING AND DESIGN, INC. PITTSBURGH (412)765-3717 POTTSTOWN (610)326-3100 LEHIGH VALLEY WWW.TRAFFICPD.COM

FIGURE 7

HARRISBURG (717)234-1430

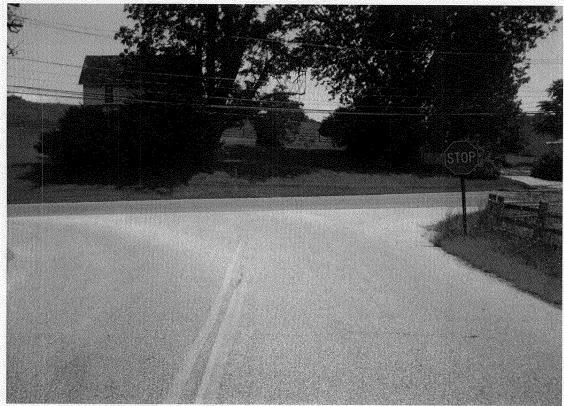
SOUTH JERSEY (856)966-4242

2020 PROJECTED CONDITIONS TRAFFIC VOLUMES

KEY:

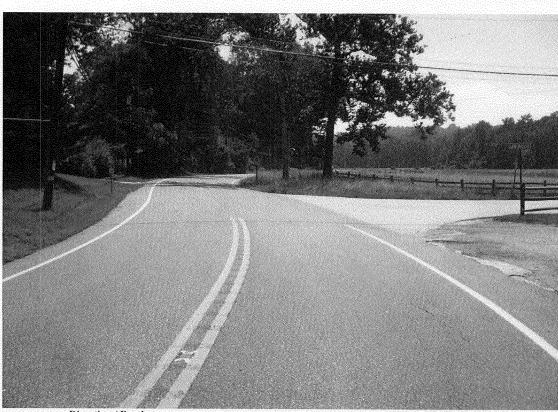
-- PROPOSED DRIVEWAY SCHEMATIC DRAWING:NOT TO SCALE

APPENDIX A STUDY AREA PHOTOGRAPHS



Direction / Road: Approach / Departure: Distance:

EB Saw Mill Road



Direction / Road: Approach / Departure: Distance:

SB Darby-Paoli Road

Approach

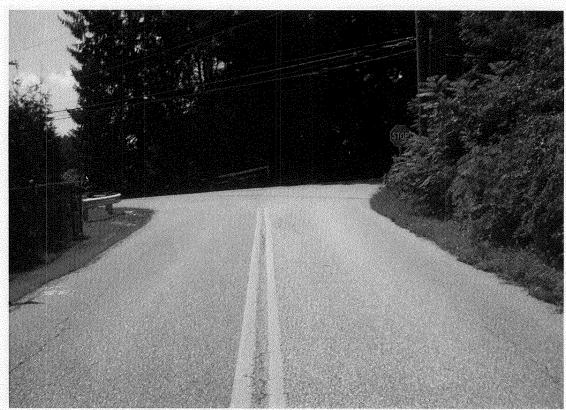




Direction / Road: Approach / Departure: Distance:

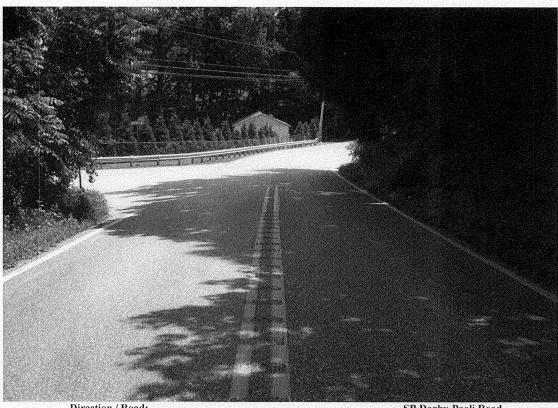
NB Darby-Paoli Road

Direction / Road: Approach / Departure: Distance:



Direction / Road: Approach / Departure: Distance:

WB Godfrey Road Approach



Direction / Road: Approach / Departure: Distance:

SB Darby-Paoli Road



Direction / Road: Approach / Departure: Distance:

NB Darby-Paoli Road

Direction / Road: Approach / Departure: Distance:

APPENDIX B TRAFFIC COUNT DATA

MANUAL COUNTS

Darby-Paoli Road & Saw Mill Road

Counter: 2

File Name: 2013-07-17 AM DPR_SMR

Counted by: M. Goodman

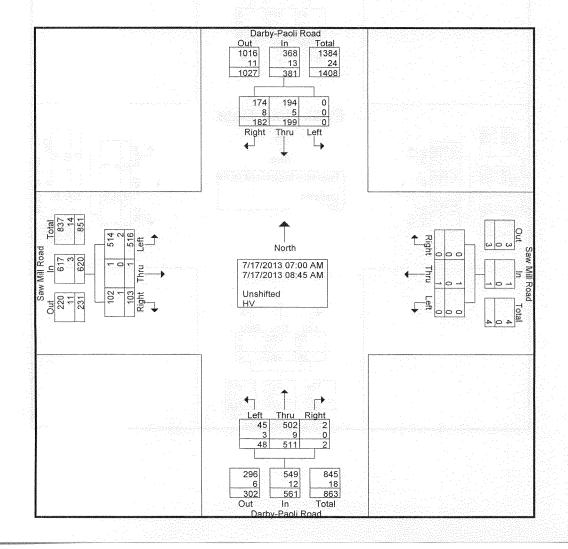
Site Code : 00000000 Start Date : 7/17/2013

Weather: Sunny

Page No : 1

Saved as: 2013-07-17 AM DPR_SMR

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100				/ Mill R astbour					/ Mill /estbo	Road und				y-Paol orthbo	i Road und				y-Paol uthbo					
enth.	Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu Total	Int. Total
	07:00 AM	41	0	6	0	47	0.	1	0	0	1	1	32	0	0	33	0	18	9	0	27	0	108	108
	07:15 AM	53	0	7	0	60	0	0	. 0	0	. 0	4	50	0	0	54	0	13	27	0	40	0	154	154
	07:30 AM	66	: 1	8	0	75	0	0	0	0	0	10	64	2	0	76	0	28	25	0	53	0	204	204
theter,	07:45 AM	61	. 0	11	0	72	0	0.:	0	0	: 0	7	66	0	0	73	0	30	24	0	54	0	199	199
	Total	221	1	32	0	254	0	1	0	0	1	22	212	2	0	236	0	89	85	0	174	0	665	665
	08:00 AM	63		177		79	۱ ۸	0	Δ	0	^	9	οΛ.			90	0	22	22		4.5	1 - 0	010	0.10
		62	:0	17	0		0	0	0	0	0	9	80	0	0	89	0	22	23	0	45	0	213	213
ibres.	08:15 AM	82	0		0	101	0	0	0	0	0	2	61	0	0	63	0	18	17	0	35	0	199	199
	08:30 AM	81	0	15	0	96	0	0	0	0	0	/	78	0	0	85	0	34	31	0	65	0	246	246
	08:45 AM	70	0	20	0	90	0	0	0	0	0	8	80	0	0	88	0	36	26	0	62	0	240	240
	Total	295	0	71	0	366	0	0	0	0	0	26	299	0	0	325	0	110	97	0	207	0	898	898
1000				102		6 (20		100				1 40	F 1 1			200		100	100		201	l .	3555 5 5 5 6 6	1.7.60
	Grand Total	516	0.2	103	0	620	0	100	0	0	1	48	511	2	0	561	0	199	182	0	381	0	1563	1563
	Apprch %	83.2	0.2	16.6		20.7	0	100	0		e e e e e e e e e e e e e e e e e e e	8.6	91.1	0.4		25.0	0	52.2	47.8		24.4			
	Total %	33	0.1	6.6		39.7	0	0.1	0		0.1	3.1	32.7	0.1	2000	35.9	0	12.7	11.6		24.4	0	100	1.50.5
beene.	Unshifted	514	100	102	0	617	0	100	0	Δ.	100	45	502	100		549	0	194	174		368	0		1535
	% Unshifted	99.6	100	99	0	99.5	0	100	0	0	100	93.8	98.2	100	0	97.9	0	97.5	95.6	0	96.6	0	0	98.2
	HV	2	0	1	0	3	0	0	0		0	3	9	0		12	0		8	Δ.	13	0	0	28
	% HV	0.4	Ð	1	0	0.5	1 0	O.	U.	. 0	0	6.2	1.8	U	0	2.1	0	2.5	4.4	0	3.4	0	0	1.8



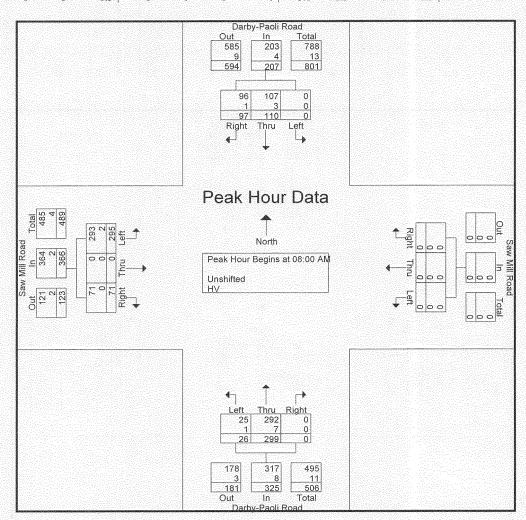
Darby-Paoli Road & Saw Mill Road

File Name : 2013-07-17 AM DPR_SMR Site Code : 00000000

Start Date : 7/17/2013

Page No : 2

		Saw Mi Easth	ll Road ound				ill Road bound		1		aoli Road bound	l	I				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right /	App. Total	Left	Thru	Right	App. Total	Int. Tota
Peak Hour Analys	is From (7:00 AM	to 08:45	AM - Pea	klofl												
Peak Hour for En	tire Inter	section E	egins at	08:00 AM	1												
08:00 AM	62	0	17	79	0	0	0	0	9	80	0	89	0	22	23	45	213
08:15 AM	82	0	19	101	0	0	0	0	2	61	0	63	0	18	17	35	199
08:30 AM	81	0	15	96	0	0	0	0	7	78	0	85	0	34	31	65	246
08:45 AM	70	0	20	90	0	0	0	0	8	80	0	88	0	36	26	62	240
Total Volume	295	0	71	366	0	0	0	0	26	299	0	325	0	110	97	207	898
% App. Total	80.6	0	19.4		0	0	0		8	92	0		0	53.1	46.9		
PHF	.899	.000	.888	.906	.000	.000	.000	.000	.722	.934	.000	.913	.000	.764	.782	.796	.913
Unshifted	293	0	71	364	0	0	0	0	25	292	0	317	0	107	96	203	884
% Unshifted	99.3	0	100	99.5	0	0	0	0	96.2	97.7	0	97.5	0	97.3	99.0	98.1	98.4
HV	2	0	0	2	0	0	0	0	1	7	0	8	0	3	1	4	14
%HV	0.7	0	0	0.5	0	0	0	0	3.8	2.3	0	2.5	0	2.7	1.0	1.9	1.6



Darby-Paoli Road & Godfrey Road

Counter: 16

File Name: 2013-07-17 AM DPR_GR

Counted by: R. Kearney

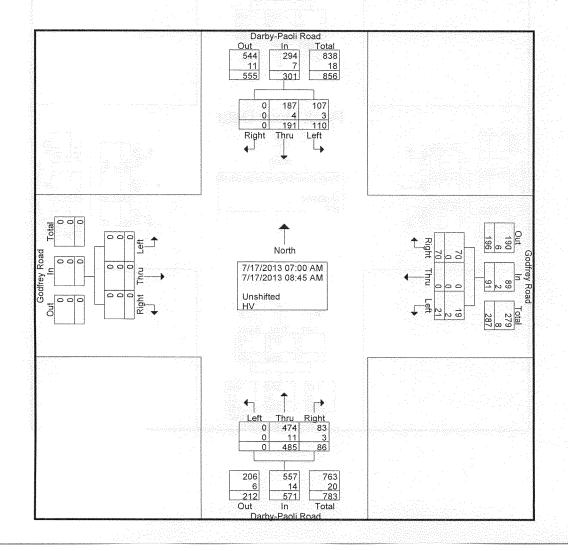
Site Code : 00000000 Start Date : 7/17/2013

Weather: Sunny

Page No : 1

Saved as: 2013-07-17 AM DPR GR

Ĩ.								3.35		Gi	roups P	rinted	Unsh	ifted -	HV									
		Godfrey Road Eastbound Time Left Thru Right RTOR App.							dfrey /estbo					y-Paol orthbo	li Road und				y-Paol outhbo					
	Start Time	Left	Thru	Right	RTOR	App. Tot	a Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu, Total	Inclu. Total	Int. Total
	07:00 AM	0.	0	0	0	() 1	0	4	0	5	0	27	6	0	33	5	21	0	0	26	0	64	64
	07:15 AM	0	0	0	0	() . 2	0	4	-0	6	0	52	9	0	61	6	12	0	0	18	0	85	85
	07:30 AM	. 0	::0	0	0	震: (0 883	. 0.:	8	0	11.	0	65	13	0	78	13	23	0	0	36	0	125	125
Min.	07:45 AM	0	0	0 -	0	() 1	0	10	0	11	0	57	14	0	71	16	29	0	0	45	0	127	127
	Total	0	0	0	0	- St () 7	0	26	0	33	0	201	42	0	243	40	85	0	0	125	0	401	401
	08:00 AM	. 0	::0	.0.	0	- 15 () 2		9	0	11	0	77	11	.0	88	18	14	0	0	32	0	131	131
Man.	08:15 AM	0	0	0	0) 2		6	0	8	0.	62	- 5	0	67	18	24	0	0	42	0	117	:- 117
	08:30 AM	0	0	0	0) 3		12	0	15	0	69	12	0	81	22	31	0	0	53	0	149	149
	08:45 AM	0	0	0	0) 7	0	17	0	24	0	76	16	0	92	12	37	0	0	49	0	165	165
	Total	0	. 0	0	0) 14	0	44	0	58	0	284	44	0	328	70	106	0	0	176	0	562	562
**																								
	Grand Total	0	-0	0 -		- () 21	0	70	0	91	0	485	86	0	571	110	191	0	0	301	0	963	963
	Apprch %	0	0	0			23.1	0	76.9		ere tildili	0		15.1			36.5	63.5	0					
	Total %	0	0	0) 2.2	0	7.3		9.4	0	***************************************	8.9	and the second	59.3	11.4	19.8	0		31.3	0	100	
990	Unshifted	0	0	0		`) 19		70		89	0	474	83		557	107	187	0		294	0	0	940
	% Unshifted	0	0	0	0		90.5		100	0	97.8	0	97.7	96.5	0	97.5	97.3	97.9	0	0_	97.7	0	0	97.6
	HV:	0	0	0	0) 2		0		2	0	11	3		14	3	4	0	^	7	0	0	23
	% HV	U	U	0	0	(9.5	0	0	0	2.2	0	2.3	3.5	. 0	2.5	2.7	2.1	0	0	2.3	0	0	2.4



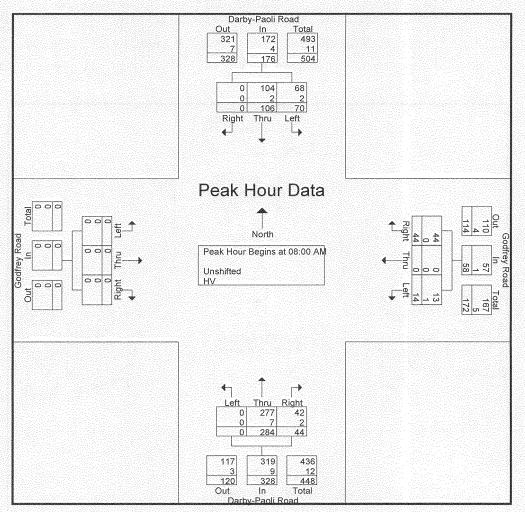
Darby-Paoli Road & Godfrey Road

File Name: 2013-07-17 AM DPR_GR

Site Code : 00000000 Start Date : 7/17/2013

Page No : 2

		Godfre Eastb	y Road ound				ey Road bound			Darby-P North	aoli Ro ibound	ad)				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Tota
eak Hour Analysi	is From 0	7:00 AM	to 08:4	5 AM - Pea	k I of I												
eak Hour for En	tire Inters	ection E	Begins at	t 08:00 AM	1												
08:00 AM	0	0	0	0	2	0	9	11	0	77	11	88	18	14	0	32	131
08:15 AM	0	0	0	0	2	0	6	8	0	62	5	67	18	24	0	42	117
08:30 AM	0	0	0	0	3	0	12	15	0	69	12	81	22	31	0	53	149
08:45 AM	0	0	0	0	7	0	17	24	0	76	16	92	12	37	0	49	165
Total Volume	0	0	0	0	14	0	44	58	0	284	44	328	70	106	0	176	562
% App. Total	0	0	0		24.1	0	75.9		0	86.6	13.4		39.8	60.2	0		
PHF	.000	.000	.000	.000	.500	.000	.647	.604	.000	.922	.688	.891	.795	.716	.000	.830	.852
Unshifted	0	- 0	0	0	13	0	44	57	0	277	42	319	68	104	0	172	548
% Unshifted	0	0	-0	0	92.9	0	100	98.3	0	97.5	95.5	97.3	97.1	98.1	0	97.7	97.5
HV	0	0	0	0	1	0	0	1	0	7	2	9	2	2	0	4	14
%HV	0	0	0	0	7.1	0	0	1.7	0	2.5	4.5	2.7	2.9	1.9	0	2.3	2.5



Darby-Paoli Road & Saw Mill Road

Counter: 18

File Name: 2013-07-17 PM DPR_SMR Site Code: 00000000

Counted by: D. Centofante

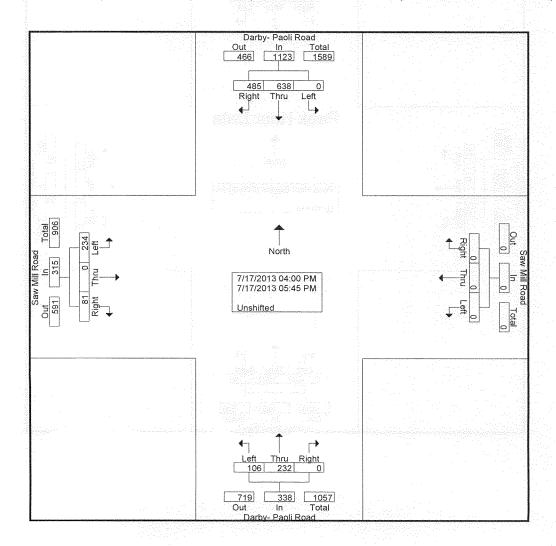
Site Code : 00000000 Start Date : 7/17/2013

Weather: Sunny

Page No : 1

Saved as: 2013-07-17 PM DPR_SMR

											Groups	s Print	ed- Ui	nshifte	d									
	Saw Mill Road Eastbound						Saw Mill Road Westbound						Darby- Paoli Road Northbound						y- Pao outhbo					
*	Start Time	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Left	Thru	Right	RTOR	App. Total	Exclu. Total	Inclu. Total	Int. Total
	04:00 PM	26	0	9	0	35	0	0	0	0	0	19	30	0	0	49	0	56	36	0	92	0	176	176
	04:15 PM	18	0	9	0	27	0	0	0	0	0.	15	21	0	1 -	36	0	61	48	- 1	109	2	172	174
	04:30 PM	22	0	16	- 1	38	0	0	0	0	0	- 5	27	0	0	32	0	83	46	2	129	3	199	202
	04:45 PM	33	0	8	1	41	0	0	0	0	0	14	25	0	1	39	0	73	63	1	136	3	216	- 219
	Total	99	0	42	2	141	0	0	0	0	0	53	103	0	2	156	0	273	193	4	466	8	763	771
	PRO L						Parties of																	
	05:00 PM	29	0	7	0	36	0	0	0	0	0	10	30	0	0	40	0	97	82	2	179	2	255	- 257
R96	05:15 PM	28	0	14	0	42	0	0	0	0	0	13	31	0	0	44	0	99	76	1	175	1	261	262
	05:30 PM	40	0	.7	S. 1.	47	0	0	0	0	0	16	35	0	1	51	0	84	79	4	163	6	261	267
-	05:45 PM	38	0	11		49	0	0	0	0	0	14	33	0	0	47	0	85	55	1	140	2	236	238
	Total	135	0	39	2	174	0	0	0	0	0	53	129	0	1	182	0	365	292	8	657	11	1013	1024
***						217						1										1		
	Grand Total	234	0	81	4	315	0	. 0	0	0	0	106	232	0	3	338	0	638	485	12	1123	19	1776	1795
	Apprch %	74.3	0				0	0.	0			31.4	68.6	0 .			0	56.8	43.2					
	Total %	13.2	0	4.6		17.7	0	0	. 0		0	6	13.1	0		19	0	35.9	27.3		63.2	1.1	98.9	



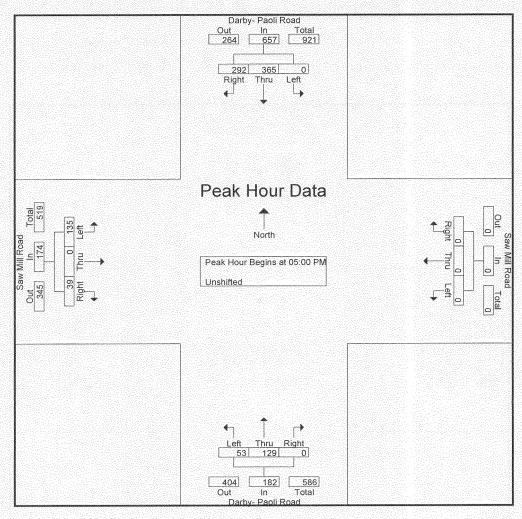
Darby-Paoli Road & Saw Mill Road

File Name: 2013-07-17 PM DPR_SMR

Site Code : 00000000 Start Date : 7/17/2013

Page No : 2

		Saw M Eastl	ill Road ound	l	Saw Mill Road Westbound						aoli Road bound		I				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right A	p. Total	Left	Thru	Right	App. Total	Int. Tota
Peak Hour Analysi	s From 0	4:00 PM	to 05:4	5 PM - Peal	c I of I												
Peak Hour for Ent	ire Inters	section E	egins a	it 05:00 PM													
05:00 PM	29	0	7	36	0	0	0	0	10	30	0	40	0	97	82	179	255
05:15 PM	28	0	14	42	0	0	0	0	13	31	0	44	0	99	76	175	261
05:30 PM	40	0	7	47	0	0	0	0	16	35	0	51	0	84	79	163	261
05:45 PM	38	0	11	49	0	0	0	0	14	33	0	47	0	85	55	140	236
Total Volume	135	0	39	174	- 0	0	0	0	53	129	0	182	0	365	292	657	1013
% App. Total	77.6	0	22.4		0	0	0		29.1	70.9	0		0	55.6	44,4		
PHF	.844	.000	.696	.888	.000	.000	.000	.000	.828	.921	.000	.892	.000	.922	.890	.918	.970



Darby-Paoli Road & Godfrey Road

Counter: 5

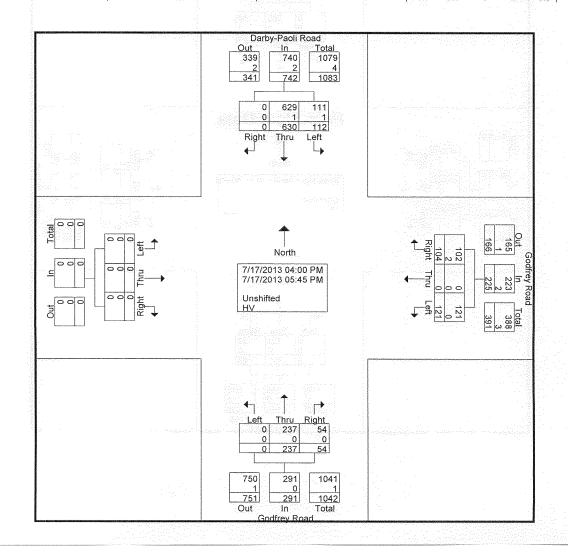
File Name: 2013-07-17 PM DPR_GR

Site Code : 00000000 Start Date : 7/17/2013

Page No : 1

Counted by: K. DiSessa
Weather: Sunny
Saved as: 2013-07-17 PM DPR_GR

Start Time Left Thru Right RTOR App. Total Left Thru Aight RTOR App. Total Left Thru Aight App. Total A	Socia, Total Incia, Total Int. Total 0 119 119
04:00 PM	0 119 119
04:15 PM	
04:30 PM	0 105 105
04:45 PM 0<	0 135 135
Total 0 <td>0 123 123</td>	0 123 123
05:00 PM	0 156 156
05:15 PM 0 0 0 0 0 18 0 13 0 31 0 27 9 0 36 14 116 0 0 130 05:30 PM 0 0 0 0 0 0 21 0 18 0 39 0 34 5 0 39 12 94 0 0 106 05:45 PM 0 0 0 0 0 0 21 0 11 0 32 0 36 5 0 41 12 82 0 0 94	0 533 533
05:15 PM 0 0 0 0 0 18 0 13 0 31 0 27 9 0 36 14 116 0 0 130 05:30 PM 0 0 0 0 0 0 21 0 18 0 39 0 34 5 0 39 12 94 0 0 106 05:45 PM 0 0 0 0 0 0 21 0 11 0 32 0 36 5 0 41 12 82 0 0 94	
05:30 PM 0 0 0 0 0 0 18 0 39 0 34 5 0 39 12 94 0 0 106 05:45 PM 0 0 0 0 0 11 0 32 0 36 5 0 41 12 82 0 0 94	0 177 177
05:45 PM 0 0 0 0 0 0 21 0 11 0 32 0 36 5 0 41 12 82 0 0 94	0 197 197
	0 184 184
Total 0 0 0 0 0 0 0 0 0 0 4444	0 167 167
	0 725 725
■ 세요리 (경찰) :	
Grand Total 0 0 0 0 0 121 0 104 0 225 0 237 54 0 291 112 630 0 0 742	0 1258 1258
Apprch % 0 0 0 53.8 0 46.2 0 81.4 18.6 15.1 84.9 0	A A .
Total % 0 0 0 0 9.6 0 8.3 17.9 0 18.8 4.3 23.1 8.9 50.1 0 59	0 100
Unshifted 0 0 0 0 121 0 102 223 0 237 54 291 111 629 0 740	0 0 1254
% Unshifted 0 0 0 0 0 0 98.1 0 99.1 0 100 100 99.2 0 0 0 99.7 HV 0 0 0 0 0 2 2 0 0 0 1 1 0 99.7	0 0 99.7
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	0 0 4



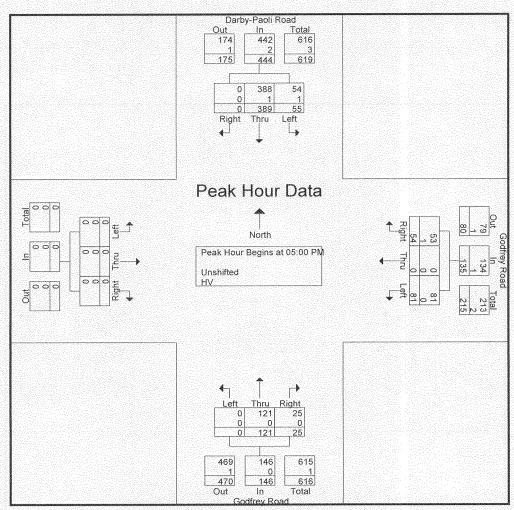
Darby-Paoli Road & Godfrey Road

File Name: 2013-07-17 PM DPR_GR

Site Code : 00000000 Start Date : 7/17/2013

Page No : 2

		Eastb	ound				ey Road bound				ey Road bound		I	Parby-P South	aoli Ro: bound	ıd	
Start Time	Left	Thru	Right	App, Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysi	s From 0	4:00 PM	to 05:4	5 PM - Peak	cl of l												
Peak Hour for Ent	ire Inter	section E	legins a	t 05:00 PM													
05:00 PM	0	- 0	0	0	21	0	12	33	0	24	6	30	17	97	0	114	177
05:15 PM	0	0	0	0	18	0	13	31	0	27	- 9	36	14	116	0	130	197
05:30 PM	0	0	0	0	21	0	18	39	0	34	5	39	12	94	0	106	184
05:45 PM	0	0	0	0	21	0	11	32	0	36	5	41	12	82	0	94	167
Total Volume	0	0	0	0	81	0	54	135	0	121	25	146	55	389	0	444	725
% App. Total	0	0	0		60	0	40		0	82.9	17.1		12.4	87.6	0		
PHF	.000	.000	.000	.000	.964	.000	.750	.865	.000	.840	.694	.890	.809	.838	.000	.854	.920
Unshifted	0	0	0	0	81	0	53	134	0	121	25	146	54	388	0	442	722
% Unshifted	0	0	0	0	100	0	98.1	99.3	0	100	100	100	98.2	99.7	0	99.5	99.6
HV	0	0	0	0	0	0	- 1	1	0	0	0	0	1	- 1	0	2	3
%HV	0	0	0	0	0	0	1.9	0.7	0	0	0	0	1.8	0.3	0	0.5	0.4



ATR COUNTS

Newtown Road WB-EB

	15-Jul-13		VB		В		bined	16-Jul-		WB		EB	Comb	
Time	Mon	A.M.	Р.М.	A.M.	P.M.	A.M.	P.M.	Tue	A.M.	P.M		. P.M.		P.M.
12:00		*	*	*	*	*	*		4	47	2	51	6	98
12:15		*	*	* * *	*	*	*		3	42	2	42	5	84
12:30		*	*	*	*	*	*		2	53	1	50	3	103
12:45		*	*	*	*	*	*		1	33	1	55	2	88
01:00		*	*	*	*	*	*		2	49	0	50	2	99
01:15		*	*	*	*	*	*		1	45	1	43	2	88
01:30		*	*	*	*	*	*		1	33	0	43	1	76
01:45		*		*		*			1	43	0	59	1.00	102
02:00 02:15		*	41 54	*	37 30	*	78		0	47	0	32	0	79
02:10		*	42	*	36	*	84 78		1	46	1	43	2	89
02:45		*	47	*	44	*-0.00	91		1 0	43 54	0 1	47	1	90
03:00		*	60	*	51	*	111		0	54 54	0	47	1 0	-101
03:15		*	70	*	42	*	112		0	53	1	43 48	1	97 101
03:30		*	52	*	34	*	86		1	60	0	40 44	1	101
03:45		*	57	*	45	*	102		Ö	64	0	47	0	111
04:00		*	75	*	54	*	129		0	51	0	50	0	101
04:15		*	102	*	58	*	160		0	95	0	44	0	139
04:30		*	90	*	47	*	137		2	99	0	55	2	154
04:45		*	97	*	46	*	143		0	78	1.	57	1	135
05:00		*	94	*	60	*	154		11	97	3	63	4	160
05:15		*	103	*	48	*	151		Ö	115	5	42	5	157
05:30		*	90	*	65	*	155		6	108	8	60	14	168
05:45		*	86	*	67	*	153		9	87	16	53	25	140
06:00		*	80	*	63	*	143		5	79	19	53	24	132
06:15		*	68	*	65	Cor x a	133		9	83	20	49	29	132
06:30		*	52	*	37	*	89		19	71	19	34	38	105
06:45		*	47	*	25	*	72		24	53	51	48	75	101
07:00		*	48	*	40	*	88		_28	48	64	34	92	82
07:15		*	32	*	32	* 171	64		33	50	. 73	23	106	73
07:30		*	40	*	29	*	69		44	27	89	25	133	52
07:45		*	32	-1 - * - 1	23	*	55		48	43	120	33	168	76
08:00		*	36	*	23	*	59	Best Delication	49	24	129	22	178	46
08:15			46	*	20	*	66		49	34	129	28	178	62
08:30		*	31	*	14	*	45		49	22	136	23	185	45
08:45		*	24	*	23	*	47		65	41	114	26	179	67
09:00		*	29	*	11	*	40		55	28	102	28	157	56
09:15		*	18	*	6	*	24		39	36	73	15	112	51
09:30		e e	16	*	6		22		37	21	55	10	92	31
09:45 10:00		*	16	*	14	*	30		29	14	62	12	91	26
10:00		*	23 7	*	6	*	29		28	15	39	14	67	29
10:13		*	7	*	4 8	*	11 15		34	24	38	13	72	37
10:30		*	7	*	1 =	*	8		49 34	10 12	35 37	4 5	84 71	14
11:00		*	15	*	4	*	19		31	11	- 31 48	5 5		17
11:15		*	6	*	- 6	*	12		32	7	46 45	7	79 77	16
11:30		*	6	*	1	*	7		41	5	33	, 5	74	14 10
11:45		*	6	*	3	*	9		37	11	47	2	84	13
Total		0	1852	0	1228	0	3080		904	2265	1620	1686	2524	3951
Day Total		185		122		308			316			306	6475	0001
% Total	V* 0.0	0.0% 6			9.9%			14		35.0%	25.0%	26.0%	J.,,	
Peak	1841 - 604	j - 96 (04:30	- (05:30	- 1	05:00	- 0		05:00	07:45	04:45	08:00	05:00
Vol.	ŭ - KW	- 1935	384	-	260	-	613	-	218	407	514	222	720	625
		A 1500	0.932	(0.970	4.444.400		0	.838	0.885	0.945	0.881	0.973	0.930

Newtown Road WB-EB

Start	17-Jul-13		WB		EB	Cı	ombined	18-Jul-		WB		EB	Co	mbined
Time	Wed	A.M		. A.M					A.M		. A.M		A.M.	P.M.
12:00		6	49	2	44	8	93		10	46	3	40	13	86
12:15		7	45	3	61	10	106		7	45	1	32	8	77
12:30		4	42	1	33	5	75		4	34	1	41	- 5	75
12:45		2	52	2	42	4	94		3	45	2	37	5	82
01:00		3	45	2	37	. 5	82		2	42	1	48	- 3	90
01:15		2	35	0	40	2	75		2	44	1	36	3	80
01:30		2	49	0	41	2	90		1	45	0	34	1	79
01:45		0	33	0	42	0	75		2	35	0	47	2	82
02:00		0	41	0	46	0	87		0	37	0	45	0	82
02:15		1	44	0	43	1	87		0	46	0	35	0	81
02:30		3	47	0	37	3	84		1	49	1	46	2	95
02:45		0	36	1	43	1	79		0	42	0	50	0	92
03:00		0	66	0	40	0	106		0	54	0	47	0	101
03:15		1	50	0	43	1	93		0	54	2	48	2	102
03:30		0	56	0	54	0	110		0	50	0	44	0	94
03:45		0	63	0	51	0-	114		1	71	0	68	1	139
04:00		1	56	1	40	2	96		0	80	0	51	0	131
04:15		0	77	. 1	44	1	121		1	75	1	51	2	126
04:30		4	99	0	46	4	145		2	84	0	46	2	130
04:45		1	111	2	63	- 3	174		0	110	2	46	2	156
05:00		2	120		58	3	178		0	114	2	56	2	170
05:15		1	88	4	54	5	142		1	118	6	70	7	188
05:30		3	105	7	68	10	173		4	103	3	46	7	149
05:45		6	82	14	73	20	155		7	108	11	62 55	18	170
06:00		7	94	17	54 50	24 36	148 138		4 8	96 78	18 22	55 54	22 30	151 132
06:15		9	88 60	27	50 47	37	107		- o 14	76 67	23	48	30 37	115
06:30		15		22		37 79	107		16	53	46	34	62	87
06:45		20	55 38	59 51	47 25	79 82	63		32	52	56	28	88	80
07:00		31 33	37	74	41	107	78 l		32 45	43	71	20 38	116	81
07:15 07:30		33 45	33	102	32	147	65		41	45	94	30	135	75
07:45		47	40	102	34	154	74		52	27	118	18	170	45
07:43		40	36	106	29	146	65		48	31	125	21	173	52
08:15		45	33	131	25 l	176	58		55	30	125	25	180	55
08:30		70	15	105	25	175	40		47	30	126	22	173	52
08:45	on and	64	32	113	33	177	65		71	34	126	26	197	60
09:00		60	22	96	22	156	44		50	27	103	20	153	47
09:15	F 1	47	34	75	23	122	57	BURNINGS	43	30	80	17	123	47
09:30		30	35	71	11	101	46		35	17	55	14	90	31
09:45		35	24	53	19	88	43		55	20	52	12	107	32
10:00		35	19	56	7	91	26		35	19	49	5	84	24
10:15		37	12	52	10	89	22		33	13	40	8	73	21
10:30		36	11	47	5	83	16		29	10	28	18	57	28
10:45		40	12	53	9	93	21		25	19	49	12	74	31
11:00		26	11	43	5	69	16		44	13	40	6	84	19
11:15		38	8	43	4	81	12		32	7	52	2	84	9
11:30		39	10	41	2	80	12		44	8	42	1	86	9
11:45		52	6	55	2	107	8		50	6	49	2	99	8
Total		950	2256	1640	1704	2590	3960		956	2306	1626	1642	2582	3948
Day Total	l	3:	206	3	344	6	550			262		268	65	30
% Total		14.5%	34.4%	25.0%	26.0%			14	1.6%	35.3%	24.9%	25.1%		
Peak	-	08:30	04:45	08:00	05:00	08:15	04:45	- 0	8:15	04:45	08:00	05:00	08:00	05:00
Vol.	-	241	424	455	253	684	667	-	223	445	502	234	723	677
P.H.F.		0.861	0.883	0.868	0.866	0.966	0.937	0	.785	0.943	0.996	0.836	0.918	0.900

Newtown Road WB-EB

Start	19-Jul-13		NΒ		В	Comb		20-Jul-		VB		В		bined
<u>Fime</u>	Fri	<u>A.M.</u>	<u>P.M.</u>	<u>A.M.</u>	P.M.	A.M.	Р.М.	Sat	A.M.	P.M.	A.M.	P.M.	A.M.	P.N
12:00		5	*	1	*	6	*		*	*	*		*	
12:15		4		2		6			*	*	*	*	*	
12:30		3	*	3	*	6	*		*	*	*	*	*	
12:45		1	*	1	*	2	*		*	*	*	*	*	
01:00		3	*	0	*	3	*		*	*	*	*	*	
01:15		1	*	1	*	2	*		*	*	*	*	*	
01:30		1	*	0	*	1	*		*	*	*	*	*	
01:45		2	*	2	*	4	*		*	*	*	*	*	
02:00		1	*	1	*	2	*		*	*	*	*	*	
02:15		2	*	0	*	2	*		*	*	*	*	*	
02:30		1	*	1	*	2	*		*	*	*	*	*	
02:45		0	*	1	*	1	*		*	*	*	*	*	
03:00		1	*	0	*	1	*		*	*	*	*	*	
03:15		0	*	1	*	1	*		*	*	*	*	*	
03:30		0	*	0	*	0	*		*	*	*	*	*	
03:45		1	*	0	*	1	*		*	*	*	*	*	
04:00		0	*	0	*	0	*		*	*	*	*	*	
04:15		1	*	0	*	1	*		*	*	*	*	*	
04:30		2	*	Ō	*	2	*		*	*	*	*	*	
04:45		1	*	3	*	4	*		*	*	*	*	*	
05:00		2	*	1	*	3	*		*	*	*	*	*	
05:15		2	*	3	*	5	*		*	*	*	*	*	
05:30		5	*	6	*	11	*		*	*	*	*	*	
05:45		4	*	16	*	20	*		*	* 55	*	*	*	
06:00		6	*	12	*	20 18	*		*	*	*	*	*	
06:15		9	*	19	*	28	*		*	*	*	*	*	
06:30		11	*	20	*	20 31	*		*	*	*	*	*	
06:45		19	*	42	*	51 61	*		*	*	*	*	*	
07:00			*	37	*		*		*	*	*	*	*	
		20 21	*		*	57	*			*			*	
07:15			*	65 05	*	86	*		*	*	*	*	*	
07:30		32	*	85	*	117	*			*	*		*	
07:45		47	*	96	*	143	*		*	*	*	*	*	
08:00		43	*	88	*	131	*					*	*	
08:15		45	*	131	*	176					•			
08:30		54		105		159	*		*	*	*	*	*	
08:45		62	*	110	*	172	*			*	*		*	
09:00		47	*	92	*	139	*		*	*	*	Alterial Control	*	
09:15		39	*	58	*	97	*		*	*			*	
09:30		25	*	65	*	90	*		in the second se	*	*	*	*	
09:45		26	*	53	*	79	*		*	*	*	*	*	
10:00		29	*	50		79	*		*	*	*	*	*	
10:15		32	*	39	*	71	*			*	*	*	*	
10:30		34	*	42		76	*			*	*	*	*	
10:45		41	*	43	*	84	*		*	×	*	*	*	
11:00		37	*	35	*	72	*		*:	*	* Constitution of the	Service **	*	
11:15		29	*	34	*	63	*		*	*	*		*	
11:30		34	*	35	*	69	*		*	*	*	- 10 1	*	
11:45		*	*	* * **	*	*	*	550,000	*	* * * * * * * * * * * * * * * * * * * *	*		*	
		785	_ 0	1399	0	2184	0		0	0	0	0	0	
Day Total		78	5	139	9	2184			0		0		0	
% Total	- 35	.9%	0.0%	64.1%	0.0%			C	0.0%	0.0%	0.0%	0.0%		
Peak	: ::: - ::::::::::::::::::::::	8:15		08:15	_	08:15	4	- 8948	erio de la compansión de Esta de la compansión de			_		
		208	1951 - 1951 참 - 1951	438	_	646	_		v				* /*** _****	
	n		3 - 749 3 3 - 75	0.836		0.918	3	Tiper. Silver	Se d			-	**************************************	
	.0	.000		0.000		V. V 1 V								

Darby-Paoli Road NB-SB

Start	15-Jul-13		NB		SB	Cr	mbined	16-Ju	_	NB		SB	Coi	mbined
Time	Mon	A.M.	P.M	. A.M		A.M		Tue	A.M		1. A.N			P.M.
12:00		*	*	*	*	*	*		0	40	3	48	3	88
12:15		*	*	*	*	*	*		- 1	56	0	43	- G-1-1	99
12:30		*	*	*	*	*	*		1	47	0	43	1	90
12:45		*	*	*	*	*	*		1	48	2	32	3	80
01:00		*	*	*	*	*	*		1	56	3	56	4	112
01:15		*	*	*	*	*	*		3	59	1	44	4	103
01:30		*	*	*	*	*	*		1	42	1	51	2	93
01:45		*	53	*	42	*	95		1	53	0	56	1	109
02:00		*	56	*	50	*	106		0	55	1	64	1	119
02:15			66	*	48	*	114		0	66	2	41	2	107
02:30		*	78	*	44	*	122		0	63	0	51	0	114
02:45		*	77	*	34	*	111		1	77	0	48	1	125
03:00		*	79	*	37	*	116		0	81	0	49	0	130
03:15		*	94	*	42	*	136		0	89	0	54	0	143
03:30		*	106	*	56	*	162		0	107	0	50	0	157
03:45		*	102	*	53	*	155		0	127	1	36	1	163
04:00		*	137	*	53	*	190		0	107	2	52	2	159
04:15		*	151	*	53	*	204		1	177	3	53	4	230
04:30		*	181	*	50	*	231		0	203	4	51	4	254
04:45		*	171	*	69	*	240		4	194	11	54	15	248
05:00		*	138	*	60	*	198		- 5	206	13	45	18	251
05:15		*	145	*	57	*	202		- 8	142	19	61	27	203
05:30		*	94	*	52	ţ	146		10	118	29	48	39	166
05:45			84	*	35		119		14	130	28	46	42	176
06:00		*	70	·	32	*	102		21	78	61	32	82	110
06:15		*	55	*	38	*	93		22	70	68	37	90	107
06:30		*	58	*	40	*	98		32	51	100	29	132	80
06:45		*	45	*	35	*	80		51	38 50	130	38	181	76 76
07:00		*	42	*	37	*	79		54	50	132	25	186	75 70
07:15		*	36	*	23	*	59		50	35	142 131	35 31	192 181	70 68
07:30		*	38	*	22	*	60		50	37			188	62
07:45		*	36 35	*	25 19	*	61 54		46 49	29 53	142 146	33 41	195	94
08:00		*		*	19	*			68	34	94	62	162	96
08:15 08:30		*	32 29	*	10 8	*	50 37		49	35	94 77	25	126	90 60
		*	18	*	8	*	26		48	29	68	28	116	57
08:45		*	13	*	12	*	25		32	17	58	22	90	39
09:00 09:15		*	19	*	7	*	26		26	16	53	20	79	36
09.13		*	11	*	3	*	14		30	17	41	19	71	36
09:45		*	9	*	6	*	15		50 50	13	33	14	83	27
10:00		*	13	*	0	*	13		38	17	46	9	84	26
10:15		*	14	*	5	*	19		42	9	45	9	87	18
10:30		*	5	*	4	*	9		37	10	54	4	91	14
10:45		*	7	*	3	*	10		41	5	38	5	79	10
11:00		*	7	*	5	*	12		43	7	49	5	92	.o 12
11:15		*	6	*	2	*	8		52	5	34	6	86	11
11:30		*	3	*	3	*	6		48	6	52	3	100	9
11:45		*	4	*	1	*	5		71	3	51	1	122	4
Total		0	2417	0	1191	0	3608		1102	3007	1968	1709	3070	4716
Day Tota	ľ		117		191	36	308		4	109	3	677	77	
% Total		0.0%	67.0%	0.0%	33.0%				14.2%	38.6%	25.3%	21.9%		
Peak	_		04:15		04:45		04:15	-	08:00	04:15	07:15	01:15	07:15	04:15
Vol.	-	•	641		238		873		214	780	561	215	756	983
P.H.F.			0.885		0.862		0.909		0.787	0.947	0.961	0.840	0.969	0.968

Darby-Paoli Road NB-SB

Start	17-Jul-13		NB		SB	Co	mbined	18-Jul-		NB		SB		bined
<u> Fime</u>	Wed	A.M				A.M.	P.M.	Thu	A.M.	P.M.	A.M		A.M.	P.M
12:00		3	61	3	52	6	113		3	49	1	39	4	8
12:15		3	55	1	53	4	108		6	61	1	36	7	(
12:30		5	49	0	42	5	91		1	45	1	46	2	
12:45		0	50	0	37	0	87		1	53	1	34	2	- 8
01:00		1	40	1	54	2	94		3	43	1	39	$\overline{4}$	{
01:15		Ò	40	0	48	0	88		Õ	43	1	47	1	(
01:30		1	57	0	35	1	92		0	48	1	33	1	
01:45		2	72	2	45	4	117		1	50	Ö	50	1	1(
02:00		0	61	0	38	0								
			and a management of the first of the con-				99		0	47	0	46	0	
02:15		0	78	0	47	0	125		0	63	1.	40	1	1(
02:30		0	67	1	46	1	113		0	63	1	42	1	1(
02:45		0	69	0	48	- 0	117		0	74	0	37	0	11
03:00		0	87	0	61	0	148		1	70	1	67	2	13
03:15		2	88	1	52	3	140		0	93	1	60	1	-15
03:30		0	112	1	48	1	160		1	120	0	45	1	16
03:45		1	130	0	41	1	171		0	113	1	48	1	16
04:00		0	145	3	68	3	213		0	154	3	51	3	20
04:15		3	188	3	64	6	252		0	170	1	47	1.	2
04:30		0	199	1	48	1	247		2	181	3	55	5	2
04:45		3	175	9	71	12	246		4	152	5 I	61		
05:00		7	144	17	80							Street Street Street Street Street	9	2
						24	224		7	156	11	78	18	2.
05:15		9	139	22	66	31	205		7	138	18	51	25	18
05:30		7	122	28	91	35	213		9	123	30	57	39	11
05:45		11	90	32	57	43	147		9	93	31	58	40	1;
06:00		23	60	58	44	81	104		21	69	61	49	82	1
06:15		25	52	69	46	94	98		31	73	54	28	85	1(
06:30		39	57	111	41	150	98		38	50	109	42	147	{
06:45		52	45	127	20	179	65		47	45	136	34	183	
07:00		47	42	120	27	167	69		52	31	132	27	184	
07:15		44	54	138	12	182	66		50	48	133	25	183	
07:30		33	42	147	18	180	60		50	41	149	16	199	
07:45		60	28	142	31	202	59		46	44	128	38	174	
08:00		57	41	153		210							COLUMN TO SERVICE STATE OF THE	
		CONTRACTOR OF THE PROPERTY OF			21		62		49	40	138	21	187	1
08:15		67	27	124	12	191	39		64	35	135	16	199	
08:30		63	32	70	12	133	44		43	32	70	20	113	,
08:45		54	37	70	20	124	57		49	31	60	20	109	
09:00		43	30	70	15	113	45		40	24	51	16	91	
09:15		45	25	61	12	106	37		33	20	47	8	80	
09:30		56	17	55	13	111	30		41	19	42	13	83	
09:45		33	17	53	9	86	26		38	13	46	20	84	,
10:00		39	11	60	6	99	17		37	16	49	10	86	
10:15		41	13	36	8	77	21		33	15	39	.3	72	
10:30		39	10	49	8	88	18		38	7	49	5	87	
10:45		47	7	50	1	97	8		50	4	45		95	
		49										3		
11:00			5	53	4	102	9		62 	6	48	1	110	
11:15		55	8	45	3	100	11		53	3	41	2	94	
11:30		66	2	54	1	120	3		55	2	52	2	107	
11:45		60	6	31	3	91	9		40	1	46	2	86	
Total	A 14	1195	2986	2071	1679	3266	4665	11	15	2871	1974	1588	3089	44
ay Total		4	181		750	79			398			562	754	
√ Total	15	5.1%	37.6%	26.1%	21.2%			14.8			26.2%	21.0%		
Peak	- 0	7:45	04:00	07:15	04:45	07:30	04:15	- 10:	45	04:15	06:45	04:45	07:30	04:
Vol.	~	247	707	580	308	783	969		20	659	550	247	759	90

Darby-Paoli Road NB-SB

Start	19-Jul-13		NB		SB -		mbined	20-Jul-		NB		SB	Comt	
Time	Fri	A.M.	P.M	. A.N	I. P.M	. A.M.	P.M.	Sat	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00		1	16	2	12	3	28		*	*	*	*	*	
12:15		3	*	0	*	3	*		*	*	*	*	*	
12:30		0	*	1	*	1	*		*	*	*	*	*	
12:45		2	*	2	*	4	*		*	*	*	•	*	
01:00		1	*	1	*	2	*		*	*	*	*	*	
			*		*	2	*		*	*	*	*	*	
01:15		0	*	2	*		ak .		*	*	*	*		
01:30		3		1		4								
01:45		1	*	1	*	2	*		*	7				
02:00		1	*	0	*	1	*		*	*	*	*	*	
02:15		1	*	0	*	1	*		*	*	*	*	*	
02:30		0	*	1	*	1	*		*	*	*	*	*	
02:45		Ō	*	0	*	0	*		*	*	*	*	*	
		0	*	0	*	0	*		*	*	*	*	*	
03:00		Samuel Same and the same	*		*		*		*	*	*	*	*	
03:15		0		0	*	0	*				*	*	*	
03:30		1		1		2								
03:45		1	*	0	*	1	*		*	*		*	*	
04:00		0	*	1	*	1	*		*	*	*	*	*	
04:15		1	*	1	*	2	*		*	*	*	*	*	
04:30		0	*	0	*	0	*		*	*	*	*	*	
04:45		2	*	8	*	10	*		*	*	*	*	*	
05:00		5	*	8	*	 13	*		*	*	*	*	*	
			*		*		*		*			*	*	
05:15		4	*	19	*	23	*				*	*		
05:30		7		23		30						Ť.		
05:45		12	*	32	*	44	*		*	•	*			
06:00		25	*	58	*	83	*		*	*	*	*	*	
06:15		22	*	- 55	*	77	*		*	*	*	*	*	
06:30		18	*	87	*	105	*		*	*	*	*	*	
06:45		34	*	122	*	156	*		*	*	*	*	*	
			*	120	*	162	*		*	*	*	*	*	
07:00		42	*		* 1		*		*		*	ak .		
07:15		50	*	121	*	171				*	*	*		
07:30		51		153		204				7				
07:45		51	*	130	*	181	*			*	*	*	•	
08:00		47	*	145	*	192	*		*	*	*	*	*	
08:15		51	*	95	*	146	*		*	*	*	*	*	
08:30		53	*	79	*	132	*		*	*	*	*	*	
08:45		43	*	67	*	110	*		*	*	*	*	*	
09:00		25	*	61	*	86	*		*	*	*	*	*	
			*		*		*		*	*	*	*	*	
09:15		31		45	*	76	*			*	*	*		
09:30		33	:1	43		76				, T				
09:45		40	*	38	*	78	*		7	*			*	
10:00		46	*	44	*	90	*		*	*	*	*	*	
10:15		40	*	57	*	97	*		*	*	*	*	*	
10:30		45	*	45	*	90	*		*	*	*	*	*	
10:45		43	*	42	*	85	*		*	*	*	*	*	
11:00	800	55	*	34	*	89	*		*	*	*	*	*	
		All the second s	*		*		*		*	*	*	*	*	
11:15		56	*	50		106	*		•	*	*	*	*	
11:30		61		43	*	104			*	*		*	*	
11:45		57	*	47	*	104	*							
Total		1065	16	1885	12	2950	28		0	0	0	0	0	
Day Tota		10	81		897	29	78		0		0		0	
% Total		5.8%	0.5%	63.3%	0.4%			- 0	.0%	0.0%	0.0%	0.0%		
Peak	7	11:00	-	07:15		07:15		•		•	-		-	
Vol.	-	229		549	-	748		-	-	-			•	
P.H.F.		0.939		0.897		0.917								

APPENDIX C VOLUME DEVELOPMENT WORKSHEETS

TPD# ESII.A.00001 7/26/2013 **Traffic Volumes Worksheet**

Intersection: Synchro Node:

Darby-Paoli Road & Sawmill Road West 0 East 0 North 0 South Adjacent intersections:

Time Period: Weekday A.M. Peak Hour

	E	astbour	nd	V	/estbou	nd	N	lorthboui	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
Existing Counts	295	0	71	0	0	0	26	299	0	0	110	97	898
Balancing													0
Existing Volumes (Balanced)	295	0	71	0	0	0	26	299	0	0	110	97	898
Base growth (0.51% compounded for 7 yrs)	11	0	3	0	0	0	1	11	0	0	4	4	34
Ardossen Farm (East Parcel)	3						1	3			6	7	20
													0
													0
2020 Base Volumes	309	0	74	0	0	0	28	313	0	0	120	108	952
New Trips							1	2					3
Redistributed Trips													0
Total Trip Distribution	0	0	0	0	0	0	1	2	0	0	0	0	3
2020 Projected Volumes	309	0	74	0	0	0	29	315	0	0	120	108	955

Time Period: Weekday P.M. Peak Hour

	E	astbour	nd .	V	Vestbou	nd	N	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
Existing Counts	135	0	39	0	0	0	53	129	0	0	365	292	1013
Balancing													0
Existing Volumes (Balanced)	135	0	39	0	0	0	53	129	0	0	365	292	1013
Base growth (0.51% compounded for 7 yrs)	- 5	0	1	0	0	0	2	5	0	0	13	111	37
Ardossen Farm (East Parcel)	8		1					8			5	5	27
	0												0
	01				<u> </u>								0
2020 Base Volumes	148	0	41	0	0	0	55	142	0	0	383	308	1077
New Trips								1			2		4
Redistributed Trips													0
Total Trip Distribution	1 0	0	1	0	0	0	0	1	0	0	2	0	4
2020 Projected Volumes	148	0	42	0	0	0	55	143	0	0	385	308	1081

TPD# ESII.A.00001 7/26/2013 Traffic Volumes Worksheet Intersection: Synchro Node:

Darby-Paoli Road & Godfrey Road

Adjacent intersections: West 0 East 0 North 0 South 0

Time Period: Weekday A.M. Peak Hour

	1 E	astbour	ıd	V	Vestboui	nd	N	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
Existing Counts	0	0	0	14	0	44	0	284	44	70	106	0	562
Balancing													0
Existing Volumes (Balanced)	0	0	0	14	0	44	0	284	44	70	106	0	562
Base growth (0.51% compounded for 7 yrs)	0	0	0	1	lo	2	0	10	2	3	4	0	22
Ardossen Farm (East Parcel)								1		1	4		6
													0
													0
2020 Base Volumes	0	0	0	15	0	46	0	295	46	74	114	0	590
New Trips		Ι				1				1			2
Redistributed Trips													0
Total Trip Distribution	0	0	0	0	0		0	0	0	1	0	0	2
2020 Projected Volumes	T 0	0	0	15	0	47	0	295	46	75	114	0	592

Time Period: Weekday P.M. Peak Hour

	E	astbour	nd l	V	Vestbour	ıd l	N	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
Existing Counts	-0	0	0	81	0	54	0	121	25	55	389	0	725
Balancing													0
Existing Volumes (Balanced)	0	0	0	81	0	54	0	121	25	55	389	0	725
Base growth (0.51% compounded for 7 yrs)	0	0	0	3	0	2	0	4	1	2	14	0	26
Ardossen Farm (East Parcel)						1		5		1	3		10
0													0
0													0
2020 Base Volumes	0	0	0	84	0	57	0	130	26	58	406	0	761
New Trips					I	1				1			2
Redistributed Trips													0
Total Trip Distribution	0	0	0	0	0	1	0	0	0	1	0	0	2
2020 Projected Volumes	0	0	0	84	0	58	0	130	26	59	406	0	763

TPD# ESII.A.00001 7/26/2013 Traffic Volumes Worksheet Intersection: Synchro Node:

Darby-Paoli Road & Site Driveway (Wheeler Field)

16 Adjacent intersections: West 0 East 0 North 0 South 0

Time Period: Weekday A.M. Peak Hour

	The second	Eastbour	nd	V	lestbour	nd	N	lorthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	Volume
Existing Counts	0	0	0	0	. 0	0	0	325	0	0	181	0	506
Balancing													0
Existing Volumes (Balanced)	0	0	0	0	0	0	0	325	0	0	181	0	506
Base growth (0.51% compounded for 7 yrs)	0	T 0	0	0	0	0	0	12	0	0	7	0	19
Ardossen Farm (East Parcel)					A			1			5		6
									10000000				0
													0
2020 Base Volumes	0	0	0	0	0	0	0	338	0	0	193	0	531
New Trips	3	Π	1 1				1						5
Redistributed Trips							7447.						0
Total Trip Distribution	3	0	1	0	0	0	1	0	0	0	0	0	5
2020 Projected Volumes	3	0	1	0	0	0	1	338	0	0	193	0	536

Time Period: Weekday P.M. Peak Hour

	l E	astbour	nd	V	Vestbou	nd	N	orthbou	nd	S	outhbou	nd	Intersection
	left	thru	right	Volume									
Existing Counts	0	0	0	0	0	0	0	182	0	0	404	0	586
Balancing													0
Existing Volumes (Balanced)	0	0	0	0	0	0	0	182	0	0	404	0	586
Base growth (0.51% compounded for 7 yrs)	0	0	0	0	0	0	0	7	0	0	14	0	21
Ardossen Farm (East Parcel)								6			4		10
)												0
													0
2020 Base Volumes] 0	0	0	0	0	0	0	195	0	0	422	0	617
New Trips	T 1		1				1					3	6
Redistributed Trips													0
Total Trip Distribution	1	0	1	0	0	0	1	0	0	0	0	3	6
2020 Projected Volumes	1 1	0	1	0	0	o l	1	195	0	0	422	3	623

APPENDIX D CAPACITY ANALYSES

EXISTING CONDITIONS

<u> </u>	``	1	1	+	7	
EBL	EBR	NBL	NBT	SBT	SBR	
۱γr			ब	1,-		
295	71	26	299	110	97	
1900	1900	1900	1900	1900	1900	
10	12	12	10	10	12	
0%			-1%	0%		
25			35	35		
649			119	180	200 to 100 to	
17.7			2.3	3.5		
0.91	0.91	0.91	0.91	0.91	0.91	
1%	0%	4%	2%	3%	1%	
		mercenna tradici Albudita.		ger er en		
Stop			Free	Free		
	295 1900 10 0% 25 649 17.7 0.91	295 71 1900 1900 10 12 0% 25 649 17.7 0.91 0.91 1% 0%	295 71 26 1900 1900 1900 10 12 12 0% 25 649 17.7 0.91 0.91 0.91 1% 0% 4%	295 71 26 299 1900 1900 1900 1900 10 12 12 10 0% -1% 25 35 649 119 17.7 2.3 0.91 0.91 0.91 0.91 1% 0% 4% 2%	Y 4 1 295 71 26 299 110 1900 1900 1900 1900 1900 10 12 12 10 10 0% -1% 0% 35 35 649 119 180 17.7 2.3 3.5 0.91 0.91 0.91 0.91 1% 0% 4% 2% 3%	295 71 26 299 110 97 1900 1900 1900 1900 1900 1900 10 12 12 10 10 12 0% -1% 0% 25 35 35 649 119 180 17.7 2.3 3.5 0.91 0.91 0.91 0.91 0.91 1% 0% 4% 2% 3% 1%

Other Area Type: Control Type: Unsignalized

Intersection Delay, s/veh	12.9							
Movement	EBL	EBR	NBL	NBT		SBT	SBR	
Vol, veh/h	295	71	26	299		110	97	
Conflicting Peds, #/hr	0	0	0	0		0	0	
Sign Control	Stop	Stop	Free	Free		Free	Free	
RT Channelized		None		None		Danis salatu gares sengas sessoas **	None	
Storage Length	0		4.00	•				
Veh in Median Storage, #	0	**		0		0		
Grade, %	0 1 1 1		•	1		0	- L	
Peak Hour Factor	91	91	91	91		91	91	
Heavy Vehicles, %	1	0	4	2		3	1	
Mvmt Flow	324	78	29	329		121	107	
Major/Minor	Minor2		Major1			Major2		
Conflicting Flow All	560	174	227	0		-1	0	
Stage 1	174	•	•	•		<u>.</u>	•	
Stage 2	386			•			•	
Follow-up Headway	3.509	3.3	2.236	#	45.55 (10) (10) (10) (10) (10) (10) (10)			
Pot Capacity-1 Maneuver	491	875	1330				-	
Stage 1	859		-	gal.		-		
Stage 2	689		10.4	(1 and • 1				
Time blocked-Platoon, %				arten singles er ar reconnere.		*		
Mov Capacity-1 Maneuver	478	875	1330			ala de la compania	10 J 1	
Mov Capacity-2 Maneuver	478	-	-					
Stage 1	859		and the					
Stage 2	670	Jesse est est est est est est est est est	•			antitutina samakanan dan mantan	•	
Approach	EB	12.57%	NB			SB		
HCM Control Delay, s	31		0.6			0		
HCM LOS	D							
Minor Lane / Major Mvmt	NBL	NBT	EBLn1	SBT	SBR		-	
Capacity (veh/h)	1330	10 11 11 12	524	1 4 A 4	*			
HCM Lane V/C Ratio	0.021		0.768					
HCM Control Delay (s)	7.766	0	31	•	20.			
HCM Lane LOS	A	Α	D					
HCM 95th %tile Q(veh)	0.066		6.836					

~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

	1	•	1	p	\	J		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	¥γ		ቕ			4		
Volume (vph)	14	44	284	44	70	106		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Width (ft)	10	12	10	12	12	10		
Grade (%)	3%		6%			-7%		
Link Speed (mph)	35		35			35		
Link Distance (ft)	1916		1868			763	Anthy pure move side (pass) removed with requirement	
Travel Time (s)	37.3		36.4			14.9		
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85		
Heavy Vehicles (%)	7%	0%	3%	5%	3%	2%		
Shared Lane Traffic (%)						THE PARTY OF THE P		
Sign Control	Stop		Free			Free		
Intersection Summary								
Aron Tuno:	Othor							

Area Type:

Other

Control Type: Unsignalized

Intersection Delay, styce 2.3	Intersection				1946.00					4000000
Vol. veh/h 14 44 284 44 70 106 Conflicting Peds, #hrr 0	Intersection Delay, s/veh	2.3								
Conflicting Peds, #fhr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Movement	WBL	WBR		NBT	NBR	SBL	SBT	100	
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Vol. veh/h	14	44		284	44	70	106		
Sign Control Stop Stop Free Free Free Free Free Free RT Channelized - None None </td <td></td> <td>0</td> <td>0</td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td></td> <td></td>		0	0		0	0	0	0		
RT Channelized - None - None - None Storage Length 0		Stop	Stop		Free	Free	Free	Free		
Storage Length			None		*	None		None		
Veh in Median Storage, # 0 - 0 - 0 Grade, % 3 - 6 - - 7 Peak Hour Factor 85 85 85 85 85 Heavy Vehicles, % 7 0 3 5 3 2 Morn Flow 16 52 334 52 82 125 Major/Minor Minor! Major Major Major Major/Minor Minor! Major Major Major Minor Minor! Major Major Stage 1 360 - - - Stage 2 714 - - - Mov Capacity-1 Maneuver 354 669 - - 1167 Stage 1 655 - - - -		0				3.54				
Grade, % 3 - 67 Peak Hour Factor 85 85 85 85 85 85 Heavy Vehicles, % 7 0 3 5 3 2 Mmnt Flow 16 52 334 52 82 125 Major/Minor Minor1 Major1 Major2 Conflicting Flow All 649 360 0 0 386 0 Stage 1 360 Stage 2 289 Follow-up Headway 3.563 3.3 - 2.227 - Pot Capacity-1 Maneuver 383 669 1167 - Stage 1 665 Stage 2 714 Time blocked-Platon, % Mov Capacity-1 Maneuver 354 669 - 1167 - Mov Capacity-2 Maneuver 354 669 1167 - Stage 1 655 Stage 1 655 Mov Capacity-2 Maneuver 354 669 1167 - Stage 1 655 Stage 1 655 Stage 1 655 Mov Capacity-2 Maneuver 354 669 1167 How Capacity-2 Maneuver 354 Stage 1 655		0	NOTES HER CONTROL OF THE CONTROL OF	NATIONAL PROPERTY OF THE PARTY.	0		***************************************	0		
Peak Hour Factor 85 80 90 85 80 90 90 90 90 90 90 90 90		3	-		6			-7		
Heavy Vehicles, % 7			85			85	85	85		
Mymit Flow 16 52 334 52 82 125 Major/Minor Minor Major1 Major2 Conflicting Flow All 649 360 0 0 386 0 Stage 1 360 - - - - - Follow-up Headway 3.563 3.3 - - 2.227 - Pot Capacity-1 Maneuver 383 669 - - 1167 - Stage 1 655 - - - - - - Stage 2 714 - <							3	2		
Major/Minor Minor1 Major1 Major2 Conflicting Flow All 649 360 0 0 386 0 Stage 1 360 -										
Conflicting Flow All 649 360 0 0 386 0 Stage 1 360										
Stage 1 360	Major/Minor	Minor1			Major1	100	Major2			
Stage 1 360 -	Conflicting Flow All	649	360		0	0	386	0		
Stage 2 289		360					***************************************			
Follow-up Headway 3.563 3.3 - 2.227 - Pot Capacity-1 Maneuver 383 669 - 11167 - Stage 1 655 Stage 2 714 Time blocked-Platoon, % Mov Capacity-1 Maneuver 354 669 - 11167 - Mov Capacity-2 Maneuver 354 Stage 1 655 Stage 2 660 Stage 2 660 Approach WB NB SB HCM Control Delay, s 12.5 0 3.3 HCM LOS B Minor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT Capacity (veh/h) 551 1167 - HCM Lane V/C Ratio 0.124 0.071 - HCM Control Delay (s) - 12.5 8.319 0 HCM Lane LOS B A A HCM 95th %tile Q(veh) 0.421 0.227 -		289	MARK SA			-	3000 - 3	1990 B.		
Pot Capacity-1 Maneuver 383 669 - 11167 - Stage 1 655		3.563	3.3		See .	98	2.227			
Stage 1 655 -							1167			
Stage 2			SENSOR REPORTS DESCRIPTION OF STREET		-	ju nominima sinas an	aratan esta esta esta esta esta esta esta esta			
Time blocked-Platoon, % Mov Capacity-1 Maneuver 354 669 - 11167 - Mov Capacity-2 Maneuver 354 Stage 1 655 Stage 2 660 Stage 2 660 Approach WB NB SB HCM Control Delay, s 12.5 0 3.3 HCM LOS B Minor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT Capacity (veh/h) 551 1167 - HCM Lane V/C Ratio 0.124 0.071 - HCM Control Delay (s) 12.5 8.319 0 HCM Lane LOS B A A HCM 95th %tile Q(veh) 0.421 0.227 -							1.0	-		
Mov Capacity-1 Maneuver 354 669 - - 1167 - Mov Capacity-2 Maneuver 354 - - - - - - Stage 1 655 - - - - - - Stage 2 660 - - - - - - Approach WB NB NB SB HCM Control Delay, s 12.5 0 3.3 HCM LOS B B Minor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT Capacity (veh/h) 551 1167 - HCM Lane V/C Ratio 0.124 0.071 - HCM Lane V/C Ratio 0.124 0.071 - HCM Control Delay (s) 0.124 0.071 - HCM Control Delay (s) 12.5 8.319 0 HCM Lane LOS - B A HCM 95th %tile Q(veh) 0.421 0.227 - TO LAND THE NEW MBL NB TO LAND THE NEW MBL NB TO LAND THE NBR WBL NB -										
Mov Capacity-2 Maneuver 354 -<		354	669				1167	- 17 <u>-</u>		
Stage 1 655 -										
Stage 2 660 -								186 IN		
Approach WB NB SB HCM Control Delay, s 12.5 0 3.3 HCM LOS B SBT Minor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT Capacity (veh/h) - - 551 1167 - - HCM Lane V/C Ratio - - 0.124 0.071 - - HCM Control Delay (s) - - 12.5 8.319 0 - HCM Lane LOS B A A - - 0.421 0.227 - HCM 95th %tile Q(veh) - - 0.421 0.227 - -					zatelianenia en est	-	-			
HCM Control Delay, s 12.5	Olago 2									
HCM Control Delay, s 12.5 0 3.3 HCM LOS B Minor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT Capacity (veh/h) - 551 1167 - HCM Lane V/C Ratio - 0.124 0.071 - HCM Control Delay (s) - 12.5 8.319 0 HCM Lane LOS B A A HCM 95th %tile Q(veh) - 0.421 0.227 -	Approach	WB			NB		SB			4.144
Minor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT Capacity (veh/h) - - 551 1167 - HCM Lane V/C Ratio - - 0.124 0.071 - HCM Control Delay (s) - - 12.5 8.319 0 HCM Lane LOS B A A HCM 95th %tile Q(veh) - - 0.421 0.227 -	CONTRACTOR OF THE PROPERTY OF	12.5			0		3.3			
Capacity (veh/h) 551 1167 - HCM Lane V/C Ratio - 0.124 0.071 - HCM Control Delay (s) - 12.5 8.319 0 HCM Lane LOS B A A HCM 95th %tile Q(veh) - 0.421 0.227 -										
Capacity (veh/h) 551 1167 - HCM Lane V/C Ratio - 0.124 0.071 - HCM Control Delay (s) - 12.5 8.319 0 HCM Lane LOS B A A HCM 95th %tile Q(veh) - 0.421 0.227 -										
HCM Lane V/C Ratio - - 0.124 0.071 - HCM Control Delay (s) - - 12.5 8.319 0 HCM Lane LOS B A A HCM 95th %tile Q(veh) - - 0.421 0.227 -			NBT NBR	THE RESERVE OF THE PERSON NAMED IN COLUMN 1						
HCM Control Delay (s) - - 12.5 8.319 0 HCM Lane LOS B A A HCM 95th %tile Q(veh) - - 0.421 0.227 -						- 1				
HCM Lane LOS B A A HCM 95th %tile Q(veh) 0.421 0.227 -			annones or antiparella richarda (per la misson)							
HCM 95th %tile Q(veh) 0.421 0.227 -										
						Α				
	HCM 95th %tile Q(veh)			0.421	0.227	1				
NISTOR	Notes									

~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

		7	1	1	+	4			
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	Ϋ́			€Î	13-				
Volume (vph)	135	39	53	129	365	292			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Lane Width (ft)	10	12	12	10	10	12			
Grade (%)	0%			-1%	0%				
Link Speed (mph)	25			35	35				
Link Distance (ft)	649			119	180		MINERAL STATE OF THE STATE OF T		
Travel Time (s)	17.7			2.3	3.5				
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97			
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%			
Shared Lane Traffic (%)							NATIONAL STATE OF THE PROPERTY	er nomen en e	
Sign Control	Stop			Free	Free				
Intersection Summary									

Area Type: Control Type: Unsignalized

Intersection Intersection Delay, s/veh	4.4											
Movement	EBL		EBR	NBL	NBT			SBT	SBR			
Vol, veh/h	135		39	53	129	1000	100	365	292			
Conflicting Peds, #/hr	0		0	0	0			0	0			
Sign Control	Stop		Stop	Free	Free			Free	Free			
RT Channelized	poveraniemo sycinos a sur un		None	•	None				None			
Storage Length	0							elle gr	100			
Veh in Median Storage, #	0			-	0			0	-			
Grade, %	0				-1			0				
Peak Hour Factor	97		97	97	97			97	97			
Heavy Vehicles, %	1		1	1	- 1			1	1			
Mvmt Flow	139		40	55	133			376	301			
Major/Minor	Minor2			Major1			1	Major2				
Conflicting Flow All	769		527	677	0				0			
Stage 1	527		-	-				7				
Stage 2	242			-				-				
Follow-up Headway	3.509		3.309	2.209				-				
Pot Capacity-1 Maneuver	371		553	919				-				
Stage 1	594		-		•			-	ises			
Stage 2	801		Asset		•				-			
Time blocked-Platoon, %								•	***			
Mov Capacity-1 Maneuver	347		553	919				-				
Mov Capacity-2 Maneuver	347		•	-	•				•			
Stage 1	594		-	-				-	-			
Stage 2	749		=						·			
Approach=	EB			NB				SB				
HCM Control Delay, s	22.7			2.7				0				
HCM LOS	C											
Minor Lane / Major Mvmt	200000000000000000000000000000000000000	NBL	NBT	EBLn1	SBT	SBR					100	
Capacity (veh/h)		919	•	379		.						
HCM Lane V/C Ratio		0.059	-	0.473								
HCM Control Delay (s)		9.165	0	22.7		naviania Piak						
HCM Lane LOS		A	Α	C								
HCM 95th %tile Q(veh)		0.189	598	2.454	*	- 10 TH-						
Notes										100		

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~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		ß			4	
Volume (vph)	81	54	121	25	55	389	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	10	12	10	12	12	10	
Grade (%)	3%		6%			-7%	
Link Speed (mph)	35		35			35	
Link Distance (ft)	1916		1868			763	
Travel Time (s)	37.3		36.4			14.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	0%	2%	0%	0%	2%	1%	
Shared Lane Traffic (%)							
Sign Control	Stop		Free			Free	

Existing Conditions
Timing Plan: PM Peak Hour

Intersection Delay, s/veh	3.6						
Movement	WBL		WBR	NBT	NBR	SBL	SBT
Vol, veh/h	81		54	121	25	55	389
Conflicting Peds, #/hr	0		0	0	0	0	0
Sign Control	Stop		Stop	Free	Free	Free	Free
RT Channelized	ngarone (So le Bearnaille en miller		Vone		None	**	None
Storage Length	0				-	14 - L	
Veh in Median Storage, #	0		м,	0	•		0
Grade, %	- 3			6			-7
Peak Hour Factor	92		92	92	92	92	92
Heavy Vehicles, %	0		2	0	0	2	- 1
Mvmt Flow	88		59	132	27	60	423
Major/Minor	Minor1			Major1		Major2	
Conflicting Flow All	687		145	0	0	159	0
Stage 1	145		mark to the state of the state		-	-	-
Stage 2	542			40000	-	_	14.7.2
Follow-up Headway	3.5		3.318			2.218	•
Pot Capacity-1 Maneuver	371		891			1420	-
Stage 1	866		the state of the s	•		•	
Stage 2	536						
Time blocked-Platoon, %				pa	-		
Mov Capacity-1 Maneuver	351		891		_	1420	
Mov Capacity-2 Maneuver	351				-	•	-
Stage 1	866			-			AM.
Stage 2	507				-		
Approach	WB	5000		NB		SB	
HCM Control Delay, s	16.3			0		0.9	
HCM LOS	С						
Minor Lane / Major Mvmt	St. Jerus II	NBT	NBR WBLn1	SBL	SBT		
		I CIVI	- 463	1420	ועט		
Capacity (veh/h)		•	central burners de conservament de la conservation de la conservation de la conservation de la conservation de				
HCM Central Delay (c)			- 0.317 - 16.3	0.042 7.647	0		
HCM Control Delay (s)			allogy objects to principal in positive in the constitutions				
HCM Lane LOS			C 200	A 0.422	A		
HCM 95th %tile Q(veh)			- 1.346	0.132			

2020 BASE CONDITIONS

	<u> </u>		•	1	1	1
				1		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	١¥			ৰ	1	
Volume (vph)	309	74	28	313	120	108
ldeal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	12	12	10	10	12
Grade (%)	0%			-1%	0%	
Link Speed (mph)	25			35	35	
Link Distance (ft)	649			119	180	
Travel Time (s)	17.7			2.3	3.5	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	1%	0%	4%	2%	3%	1%
Shared Lane Traffic (%)						
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	t					

Intersection Delay, s/veh	16.3									
Movement	EBL		EBR	NBL	NBT		SBT	SBR		
Vol, veh/h	309		74	28	313	1 may 1 may 2 may	120	108		
Conflicting Peds, #/hr	0		0	0	0		0	0		
Sign Control	Stop		Stop	Free	Free		Free	Free		
RT Channelized	OND-CHARACTERS SERVICE BENCHMAN		None		None			None		
Storage Length	0			-	_					
Veh in Median Storage, #	0		•	-	0	MILLEUM EUROPES (HER OPHILLE SONDE LE STANDENS SO	0	posestabelistorosinensii		
Grade, %	0			-	-1		0			
Peak Hour Factor	91		91	91	91		91	91		
Heavy Vehicles, %	1		0	4	2		3	1		
Vivmt Flow	340		81	31	344		132	119		
Major/Minor	Minor2			Major1			Major2			
Conflicting Flow All	596		191	251	0			0		
Stage 1	191						PERSONAL PROPERTY.			
Stage 2	405						<u>.</u>			
Follow-up Headway	3.509		3.3	2.236		eriotika kan ne an keliler kan menjalik memenin njiren superbisu	maning graphs and a new pro-	ugaringkandangsading		
Pot Capacity-1 Maneuver ■	468		856	1303						
Stage 1	844		A COLUMN ACTION	-	-			-		
Stage 2	676				<u>.</u>					
Time blocked-Platoon, %					-	antipologic metalogica polograpia Anadalina kili gili. N.A. M. Milliona dan germadik (1) da		### ##################################		
Mov Capacity-1 Maneuver	454		856	1303						
Mov Capacity-2 Maneuver	454						-			
Stage 1	844			÷						
Stage 2	656		BERTHANIAN AND ARREST		# subsupprocessors		-		10000000000000000000000000000000000000	
Approach	EB			NB			SB			
HCM Control Delay, s	40.1			0.6			0			
HCM LOS	E									
Minor Lane / Major Mvmt		NBL	NBT	EBLn1	SBT	SBR				
Capacity (veh/h)		1303		499						
HCM Lane V/C Ratio		0.024		0.843						
HCM Control Delay (s)		7.83	0	40.1		用推翻表表表				
HCM Lane LOS		Α	Α	Е						
HCM 95th %tile Q(veh)		0.073	111-1	8.596						
Votes										

	1	•	1	-	1	ļ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Ϋ́		4			4	
Volume (vph)	15	46	295	46	74	114	
ldeal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	10	12	10	12	12	10	
Grade (%)	3%		6%			-7%	
Link Speed (mph)	35		35			35	
Link Distance (ft)	1916		1868			763	
Travel Time (s)	37.3		36.4			14,9	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	
Heavy Vehicles (%)	7%	0%	3%	5%	3%	2%	
Shared Lane Traffic (%)							The American Control of Control o
Sign Control	Stop		Free			Free	

Control Type: Unsignalized

ntersection Delay, s/veh	2.4								
Movement	WBL	Marie de la companya	BR		NBT	NBR	SBL	SBT	
√ol, veh/h	15		46	de al	295	46	74	114	
Conflicting Peds, #/hr	0		0		0	0	0		
Sign Control	Stop	Ç	top		Free	Free	Free	Free	
RT Channelized	erija a XIM E lesis -		one			None		None	
Storage Length	0						4		
eh in Median Storage, #	0		ingening property		0	9 (19 (19 (19 (19 (19 (19 (19 (19 (19 (1	- -	0	
Grade, %	3				6			-7	
eak Hour Factor	85		85		85	85	85	85	2018-2019 S.
leavy Vehicles, %	7		0		3	5	3	2	
/lvmt Flow	18		54		347	54	87	134	SECONOMINATION CONTRACTOR INTERPRETATION OF THE STATE OF THE PROPERTY OF THE STATE
Major/Minor	Minor1			a Light British	Major1		Major2	New York	
Conflicting Flow All	682		374		0	0	401	0	
Stage 1	374								
Stage 2	308					_	in 1990 and the second		
ollow-up Headway	3.563	Harding series of the control of the	3.3				2.227		
ot Capacity-1 Maneuver	364		356			3	1152		
Stage 1	643								
Stage 2	697							-	
ime blocked-Platoon, %					-	-			
Nov Capacity-1 Maneuver	334		356				1152		
Nov Capacity-2 Maneuver	334	COMPILE HIR PRESIDENCE						-	
Stage 1	643				-				
Stage 2	640		-		jen.	Para Santa San			
pproach	WB				NB		SB		
ICM Control Delay, s	12.9				0		3.3		
ICM LOS	В				ARRINAL TARREST HAT THE STATE OF		esciences (II operations)		
Minor Lane / Major Mvmt		NBT N	BR	WBLn1	SBL	SBT			
apacity (veh/h)	127 SH 1			530	1152				
ICM Lane V/C Ratio				0.135	0.076	-			
ICM Control Delay (s)				12.9	8.38	0			
ICM Lane LOS				В	A	A			
ICM 95th %tile Q(veh)				0.466	0.245				
					mu pinipagnak (elipa	menglivitigati (Alba Cayo			ne zamen kuntur zen ez ez eta erre ez en erren ez eta erren eta eta en en en en en en erren eta erren eta erre Erren erren er

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Timing Plan: PM Peak Hour

	•	``	1	1	Į.	1		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	W			स	ቕ			
Volume (vph)	148	41	- 55	142	383	308		
ldeal Flow (vphpl)	1900	1900	1900	1900	1900	1900	habetanger/schilderbriede de 1990/00/2002/schilder 1992/1992/schilderbriede 1992/schilder 1992/schilderbriede 1992/schilderbri	
Lane Width (ft)	10	12	12	10	10	12		
Grade (%)	0%			-1%	0%			
Link Speed (mph)	25			35	35			
Link Distance (ft)	649			119	180			
Travel Time (s)	17.7			2.3	3.5			
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97		
Heavy Vehicles (%)	1%	1%	1%	1%	1%	1%		
Shared Lane Traffic (%)								
Sign Control	Stop			Free	Free			
Intersection Summary								
Area Type:	Other							

Control Type: Unsignalized

Intersection											
ntersection Delay, s/veh	5.2										
Movement	EBL		EBR	NBL	NBT		SE	aT.	SBR		
/ol, veh/h	148		41	55	142			33	308	10000	
Conflicting Peds, #/hr	0		0	0	0			0	0		
Sign Control	Stop		Stop	Free	Free		Fre	e	Free		
RT Channelized			None	-	None			e e e e e e e e e e e e e e e e e e e	None		
Storage Length	0			58-66-5	•			-	•		
/eh in Median Storage, #	0			-	0			0	**		
Grade, %	0				-1			0	+ 1		
Peak Hour Factor	97		97	97	97)	97	97		
Heavy Vehicles, %	1		1	1	1			1	1		
Avmt Flow	153		42	57	146		3(95	318		
Major/Minor	Minor2			Major1			Majo	r2			
Conflicting Flow All	814		554	712	0		77.59	-	0		
Stage 1	554				edino, ma⊼ana •			-	•		
Stage 2	260								- 1		
Follow-up Headway	3.509		3.309	2.209	en e				-		
ot Capacity-1 Maneuver	349		534	892	1			•			
Stage 1	577		**	•	_						
Stage 2	786		0.9142								
Γime blocked-Platoon, %								•	•		
Mov Capacity-1 Maneuver	325		534	892				•			
Nov Capacity-2 Maneuver	325		-	-				es suportarione	#		
Stage 1	577							•			
Stage 2	731		essancio sulcino reginar	-	provinces strength in secure						
Approach	EB			NB			8	SB _	60 (E. J.) (19		
HCM Control Delay, s HCM LOS	26.8 D			2.6				0			
IGM COS											
Minor Lane / Major Mvmt		NBL	NBT	EBLn1	SBT	SBR					
Capacity (veh/h)		892		355	•						
ICM Lane V/C Ratio		0.064	enrosere constant	0.549							
HCM Control Delay (s)		9.31	0	26.8							
ICM Lane LOS		A	A	D							
ICM 95th %tile Q(veh)		0.203		3.153	*						
lotes											

~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

	~	•	1	~	\		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	W		14			क	
Volume (vph)	84	57	130	26	58	406	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	10	12	10	12	12	10	
Grade (%)	3%		6%			-7%	
Link Speed (mph)	35		35			35	
Link Distance (ft)	1916		1868			763	
Travel Time (s)	37.3		36.4			14.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	0%	2%	0%	0%	2%	1%	
Shared Lane Traffic (%)			entros como ACRARION II				
Sign Control	Stop		Free			Free	

Area Type: Other Control Type: Unsignalized

Intersection								
Intersection Delay, s/veh	3.8							
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Vol, veh/h	84	57	130	26	58	406		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized		None		None	=	None		
Storage Length	0			-	-			
Veh in Median Storage, #	0		0	-	entantin semenanan	0		
Grade, %	3	60 (10 to 10 to	6		100	-7		
Peak Hour Factor	92	92	92	92	92	92		
Heavy Vehicles, %	0	2	0	0	2	- 1		
Mvmt Flow	91	62	141	28	63	441		
Major/Minor	Minor1	7.2	Major1		Major2			
Conflicting Flow All	722	155	0	0	170	0		
Stage 1	155	A CONTRACTOR CONTRACTOR STATE OF THE STATE O	Principals (Silvery) as a six of the six of			-		
Stage 2	567			1		4 ()		
Follow-up Headway	3.5	3.318	en en Decembra de La Septimbra de Marca de Constituir de La Septimbra de La Septimbra de La Septimbra de La Se La Constituir de La Constituir de La Constituir de La Septimbra de La Septimbra de La Septimbra de La Septimbra	-	2.218		**************************************	
Pot Capacity-1 Maneuver	352	879		-	1407	•		
Stage 1	856		esta pella e Locatia se della si statica di periodi e terra il		······································	Marketin strategies		
Stage 2	520		-	•		- 1		
Time blocked-Platoon, %						-		
Mov Capacity-1 Maneuver	331	879			1407	an Ta		
Mov Capacity-2 Maneuver	331			•	-	-		
Stage 1	856							
Stage 2	489							
Approach	WB		NB	9.4	SB			
HCM Control Delay, s	17.4				1			
HCM LOS	C							
Minor Lane / Major Mvmt		NBT NBR W	/BLn1 SBL	SBT				
Capacity (veh/h)			443 1407					
HCM Lane V/C Ratio			0.346 0.045	-				
HCM Control Delay (s)			17.4 7.679	0				
HCM Lane LOS			C A	Α				
HCM 95th %tile Q(veh)			1.523 0.141					
Notes			100					

~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

2020 PROJECTED CONDITIONS

Timing Plan: AM Peak Hour

	<u> </u>	``	1	1	1	7	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			ঝ	ĵ _è		
Volume (vph)	309	74	29	315	120	108	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	10	12	12	10	10	12	
Grade (%)	0%			-1%	0%		
Link Speed (mph)	25			35	35		
Link Distance (ft)	649			119	180		
Travel Time (s)	17.7			2.3	3.5		
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	
Heavy Vehicles (%)	1%	0%	4%	2%	3%	1%	
Shared Lane Traffic (%)		regular pegelőkőtészetés					
Sign Control	Stop			Free	Free		

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Timing Plan: AM Peak Hour

Intersection										
Intersection Delay, s/veh	16.7									
Movement	EBL		EBR	NBL	NBT			BT	SBR	
Vol, veh/h	309		74	29	315			20	108	
Conflicting Peds, #/hr	0		0	0	0.0			0	0	
Sign Control	Stop		Stop	Free	Free		Fr	ee	Free	
RT Channelized			None		None			an unional	None	
Storage Length	0							-	-	
Veh in Median Storage, #	0				0			0		
Grade, %	0		•		-1			0		
Peak Hour Factor	91		91	91	91			91	91	
Heavy Vehicles, %	1		0	4	2			3	1	
Mvmt Flow	340		81	32	346		1	32	119	
Major/Minor	Minor2			Major1			Majo	vr2		
Conflicting Flow All	601		191	251	0		wich		0	
Stage 1	191		101	- LUI	•			_	Mile de Marci	
Stage 2	410									
Follow-up Headway	3.509		3,3	2.236						
Pot Capacity-1 Maneuver	465		856	1303				_		
Stage 1	844		na MMX a					-	-	
Stage 2	672		-							
Time blocked-Platoon, %								•		
Mov Capacity-1 Maneuver	451		856	1303				L		
Mov Capacity-2 Maneuver	451		-	eallant (at ann a leannaidh	albur erez sillen erezen			**	-	
Stage 1	844								1	
Stage 2	652		mental control of the	grantzanini razonias				•		
Approach	EB			NB				SB	F (4.85)	
HCM Control Delay, s	40.9			0.7				0		
HCM LOS	E									
Minor Lane / Major Mvmt		NBL	NBT	EBLn1	SBT	SBR				
Capacity (veh/h)		1303		496	-	100				
HCM Lane V/C Ratio	and a management	0.024	-	0.849	SEARCHESTER (CONTRACTOR CANDES	antan international acception				
HCM Control Delay (s)		7.832	0	40.9	125					
HCM Lane LOS		Α	Α	E						
HCM 95th %tile Q(veh)		0.075		8.717	-					
Notes										

HCM 2010 TWSC
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~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Tim	ina Pl	an: AN	Peak Hour

	1	٨.	†	~	\	Ţ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		1≯			व	
Volume (vph)	15	47	295	46	75	114	
ldeal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	10	12	10	12	12	10	
Grade (%)	3%		6%			-7%	
Link Speed (mph)	35		35			35	
Link Distance (ft)	1916		1868			763	
Travel Time (s)	37.3		36.4			14.9	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	
Heavy Vehicles (%)	7%	0%	3%	5%	3%	2%	
Shared Lane Traffic (%)					SASTERNA SERVICIONES ESTADOS COM		
Sign Control	Stop		Free			Free	

Area Type: Other Control Type: Unsignalized

Novement WBL WBR NBT NBR SBL SBT	ntersection									
ol, veh/h	ntersection Delay, s/veh	2.4								
onflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Movement	WBL	WBR	N	ВТ	NBR	SBL	SBT		
Stop Stop Free Free Free Free Free T Channelized - None - None - None - None - None - None - None - None - None - None - None - None - None - None - None - None - None	/ol, veh/h	15	47	2	95	46	75	114		
Stop Stop Free Free Free Free Free T Channelized - None - None		0	0		0	0	0	0		
T Channelized - None - None torage Length 0	Sign Control	Stop	Stop	Fr	ee	Free	Free	Free		
eh in Median Storage, # 0 - 0 - 0 0 - 7 0 0 7 7 7 0 7 7 7 7 7 8 8 7 8 7 8 7 8	T Channelized		None		•	None	-	None		
eh in Median Storage, # 0 - 0 - 0 0 - 7 0 0 7 7 7 0 7 7 7 7 7 8 8 7 8 7 8 7 8		0								
rade, % 3 - 6 77 eak Hour Factor 85 85 85 85 85 85 eavy Vehicles, % 7 0 3 5 3 2 lvmt Flow 18 55 347 54 88 134 lajor/Minor Minor1 Major2 onflicting Flow All 685 374 0 0 401 0 Stage 1 374 Stage 2 311 Stage 2 311 1152 - ot Capacity-1 Maneuver 362 656 - 1152 - Inor Blocked-Platoon, % lov Capacity-1 Maneuver 332 656 - 1152 - lov Capacity-1 Maneuver 332 656 1152 - Stage 1 643 Stage 1 643 Stage 2 695 Incorporation Maneuver 332 656 1152 - Stage 1 643 Stage 1 643 Stage 1 643 Stage 2 695 Incorporation Maneuver 332 656 1152 Stage 1 643 Stage 1 643 Stage 2 637 Stage 1 643 Stage 2 637 Stage 1 643		0			0	_	-	0		
eak Hour Factor 85 85 85 85 85 85 eavy Vehicles, % 7 0 3 5 3 2 eavy Vehicles, % 7 0 3 5 3 2 eavy Vehicles, % 7 0 3 5 3 2 eavy Vehicles, % 7 0 3 5 3 2 eavy Vehicles, % 7 0 3 5 3 2 eavy Vehicles, % 7 0 0 3 5 3 5 3 2 eavy Vehicles, % 7 0 0 3 47 54 88 134 eavy Vehicles, % 134 eavy Vehi		3			6	-	-	-7		
eavy Vehicles, % 7 0 3 5 3 2 vmt Flow 18 55 347 54 88 134 ajor/Minor Minor1 Major1 Major2 onflicting Flow All 685 374 0 0 0 401 0 Stage 1 374 Stage 2 311 Ollow-up Headway 3.563 3.3 - 2.227 - ot Capacity-1 Maneuver 362 656 - 1152 - Stage 1 643 ov Capacity-1 Maneuver 332 656 - 1152 - ov Capacity-1 Maneuver 332 656 - 1152 - ov Capacity-1 Maneuver 332 656 152 - ov Capacity-2 Maneuver 332 656 152 - Stage 1 643 Stage 1 643 Stage 1 643 Ov Capacity-2 Maneuver 332 Stage 1 643 Stage 2 637 Stage 2 637 Stage 2 637 Stage 2 637 Opproach WB NB SB CM Control Delay, s 12.9 0 3.3 CM LOS B			85		85	85	85	85		
Number N			0		3	- 5	3	2		
Stage 1 374				3		54	88	134		
onflicting Flow All 685 374 0 0 401 0 Stage 1 374 - - - - Stage 2 311 - - - - ollow-up Headway 3.563 3.3 - - 2.227 - ot Capacity-1 Maneuver 362 656 - - 1152 - Stage 1 643 - - - - - Iov Capacity-1 Maneuver 332 656 - - 1152 - Iov Capacity-2 Maneuver 332 - - - - - Stage 1 643 - - - - - Stage 2 637 - - - - - Stage 2 637 - - - - - Approach WB NB SB CM Control Delay, s 12.9 0 3.3 CM Los B B SB Inor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT apacity (veh/h) - - 531 1152 - - CM Lane V/C Ratio <td>ajor/Minor</td> <td>Minort</td> <td></td> <td>Mai</td> <td>vr1</td> <td></td> <td>Majora</td> <td></td> <td></td> <td></td>	ajor/Minor	Minort		Mai	vr1		Majora			
Stage 1 374 -			37/	iviaje				Λ		
Stage 2 311 -										
Stage 1										
of Capacity-1 Maneuver 362 656 1152 - Stage 1 643										
Stage 1 643 -										
Stage 2 695 - - - - - Ime blocked-Platoon, % -										
Ime blocked-Platoon, % lov Capacity-1 Maneuver 332 656 - 1152 - lov Capacity-2 Maneuver 332 Stage 1 643 Stage 2 637 Stage 2 637 Pproach WB NB SB CM Control Delay, s 12.9 0 3.3 CM LOS B Inor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT apacity (veh/h) 531 1152 - CM Lane V/C Ratio - 0.137 0.077 -										
lov Capacity-1 Maneuver 332 656 - - 1152 - lov Capacity-2 Maneuver 332 - <td></td> <td>030</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>		030								
Stage 1		222	GEG				1150			
Stage 1 643 -										
Stage 2 637 -										
pproach WB NB SB CM Control Delay, s 12.9 0 3.3 CM LOS B Inor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT apacity (veh/h) - 531 1152 - CM Lane V/C Ratio - 0.137 0.077 -										
CM Control Delay, s 12.9 0 3.3 CM LOS B Inor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT apacity (veh/h) - 531 1152 - CM Lane V/C Ratio - 0.137 0.077 -	Staye 2	007								
CM LOS B linor Lane / Major Mvmt	pproach	WB			VB		SB		- 20.21	
linor Lane / Major Mvmt NBT NBR WBLn1 SBL SBT apacity (veh/h) 531 1152 - CM Lane V/C Ratio - 0.137 0.077 -	ICM Control Delay, s				0		3.3			
apacity (veh/h) 531 1152 - CM Lane V/C Ratio 0.137 0.077 -	CM LOS	В								
apacity (veh/h) 531 1152 - CM Lane V/C Ratio 0.137 0.077 -	Minor Lane / Major Mymt		NBT NBR \	WBLn1 S	BL	SBT				
CM Lane V/C Ratio 0.137 0.077 -									-	
CM Lane LOS B A A	CM Lane LOS									
	ICM 95th %tile Q(veh)									
and the state of t	lotes					autorio (1995) Vi				

~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error: Computation Not Defined

Timing Plan: AM Peak Hour

	•	\rightarrow	1	1	1	1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W			स	12-		
Volume (vph)	3	1	1	338	193	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
_ane Width (ft)	12	12	12	10	10	12	
Grade (%)	0%			-1%	-2%		
Link Speed (mph)	25			35	35		
Link Distance (ft)	378			763	712	Mariana Arra Car (Million Astrollar (Taxon)	
Fravel Time (s)	10.3			14.9	13.9		
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	
Heavy Vehicles (%)	2%	2%	2%	3%	2%	2%	
Shared Lane Traffic (%)		escar esta esta 15 170 (15 10 10 10 10 10 10 10 10 10 10 10 10 10				AND DESCRIPTION OF THE PROPERTY OF THE PROPERT	
Sign Control	Stop			Free	Free		

Area Type: Control Type: Unsignalized

Timing Plan: AM Peak Hour

Intersection					112				
Intersection Delay, s/veh	0.1								
Movement	EBL		EBR	NBL	NBT		SBT	SBR	
Vol, veh/h	3		1	- 1	338		193	0	
Conflicting Peds, #/hr	0		0	0	0		0	0	
Sign Control	Stop		Stop	Free	Free		Free	Free	
RT Channelized	-		None	-	None			None	
Storage Length	0			2.0					
Veh in Median Storage, #	0		·		0		0	-	
Grade, %	0		Marie	14 1/2	-1		-2		
Peak Hour Factor	88		88	88	88		88	88	
Heavy Vehicles, %	2		2	2	3		2	2	
Mvmt Flow	3		1	1	384		219	0	
Major/Minor	Minor2			Major1	194		Major2		
Conflicting Flow All	605		219	219	0			0	
Stage 1	219		•	•	_		•	•	
Stage 2	386							1.50	
Follow-up Headway	3.518		3.318	2.218	-		•	-	
Pot Capacity-1 Maneuver	461		821	1350					
Stage 1	817		-		•		-	-	
Stage 2	687							1114	
Time blocked-Platoon, %					•		-	•	
Mov Capacity-1 Maneuver	461		821	1350				1.0	
Mov Capacity-2 Maneuver	461		44	-	-		-		
Stage 1	817			-					
Stage 2	686		•	-	-		•		
Approach	EB			NB			SB		
HCM Control Delay, s	12			0			0		
HCM LOS	В								
Minor Lane / Major Mvmt		NBL	NBT	EBLn1	SBT	SBR			
Capacity (veh/h)		1350		518					
HCM Lane V/C Ratio		0.001		0.009					
HCM Control Delay (s)		7.669	0	12					
HCM Lane LOS		Α	A	В					
HCM 95th %tile Q(veh)		0.003		0.027		5			
Notes									

~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

	•	N	•	1	1	7	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W			स	1>		
Volume (vph)	148	42	55	143	385	308	
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
ane Width (ft)	10	12	12	10	10	12	
Grade (%)	0%			-1%	0%		
ink Speed (mph)	25			35	35		
ink Distance (ft)	649			119	180		
Travel Time (s)	17.7			2.3	3.5		
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
leavy Vehicles (%)	1%	1%	1%	1%	1%	1%	
Shared Lane Traffic (%)							
Sign Control	Stop			Free	Free		

Intersection Summary

Area Type: Other

Control Type: Unsignalized

Timing Plan: PM Peak Hour

Intersection Intersection Delay, s/veh	5.2								CONTRACT.
Movement	EBL	EBR	NBL	NBT		SBT	SBR		
Vol, veh/h	148	42	55	143		385	308		
Conflicting Peds, #/hr	0		0	0		0	0		
Sign Control	Stop	Stop	Free	Free		Free	Free		
RT Channelized	AND AND PARTY OF THE PROPERTY OF THE PARTY O	None	-	None			None		
Storage Length	0		-	100			-		
Veh in Median Storage, #	0		•	0		0	e guos entrepris de sus sus sentres.		
Grade, %	0		-	-1		0	100		
Peak Hour Factor	97	97	97	97		97	97		
Heavy Vehicles, %		-11	1	1		1	1		
Mvmt Flow	153	43	57	147		397	318		
Major/Minor	Minor2		Major1			Major2			
Conflicting Flow All	817	556	714	0			0		
Stage 1	556		•			-			
Stage 2	261		-			4	14 G-11		
Follow-up Headway	3.509	3.309	2.209	•					
Pot Capacity-1 Maneuver	347	533	891				-		
Stage 1	576	-	-	-		-	-		
Stage 2	785		- 4						
Time blocked-Platoon, %				-		-	-		
Mov Capacity-1 Maneuver	323	533	891						
Mov Capacity-2 Maneuver	323	•		-		-	-		
Stage 1	576						•		
Stage 2	730	-		-		•	•		
Approach	EB		NB		1000	SB		2000 B	
HCM Control Delay, s	27.1		2,6			0			
HCM LOS	D								
Minor Lane / Major Mvmt	NE	3L NBT	EBLn1	SBT	SBR				
Capacity (veh/h)	89	91 -	354		- (41)		mark at all		
HCM Lane V/C Ratio	0.0	34 -	0.553	•					
HCM Control Delay (s)	9.3	15 0	27.1						
HCM Lane LOS		A A	D						
HCM 95th %tile Q(veh)	0.20	03	3.199						
Notes									AMERICANI EMBERSANI

~: Volume Exceeds Capacity; \$: Delay Exceeds 300 Seconds; Error : Computation Not Defined

Timing Plan: PM Peak Hour

	1	•	1	1	\ -	Į.	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	W		4			क	
Volume (vph)	84	58	130	26	59	406	
ldeal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	10	12	10	12	12	10	
Grade (%)	3%		6%			-7%	
Link Speed (mph)	35		35			35	
Link Distance (ft)	1916		1868			763	
Travel Time (s)	37.3		36.4			14.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	0%	2%	0%	0%	2%	1%	
Shared Lane Traffic (%)							
Sign Control	Stop		Free			Free	

Area Type: Control Type: Unsignalized Other

Intersection Intersection Delay, s/veh	3.9								
Movement	WBL	WBR	NBT	NBR	SBL	SBT			
Vol, veh/h	84	58	130	26	59	406			100
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized		None		None		None			
Storage Length	0								
Veh in Median Storage, #	0		0			0			
Grade, %	3		6			-7			
Peak Hour Factor	92	92	92	92	92	92			
Heavy Vehicles, %	0	2	0	0	2	1			
Mvmt Flow	91	63	141	28	64	441			
Major/Minor	Minor1		Major1		Major2				
Conflicting Flow All	725	155	0	0	170	0			
Stage 1	155	-	as a	*		•			
Stage 2	570								
Follow-up Headway	3.5	3.318	And the second s	•	2.218				
Pot Capacity-1 Maneuver	350	879		•	1407				
Stage 1	856	Marie Control	198						
Stage 2	518		10.20 E		•	1			
Time blocked-Platoon, %			#			-			
Mov Capacity-1 Maneuver	329	879			1407	-			
Mov Capacity-2 Maneuver	329	###		-		-			
Stage 1	856			-	-				
Stage 2	487				960	-			
			110		O D			Alexand.	
Approach Approach	WB		NB NB		SB				
HCM Control Delay, s	17.5		0		1				
HCM LOS	C								
Minor Lane / Major Mvmt	NI	BT NBR WB	Ln1 SBL	SBT					
Capacity (veh/h)			442 1407						
HCM Lane V/C Ratio			349 0.046	e de la companya de La companya de la companya de l					
HCM Control Delay (s)			7.681	0					
HCM Lane LOS			C A	Ā					
HCM 95th %tile Q(veh)		2/4/41/5-2/5-1.	543 0.143						
AND THE PERSON NAMED OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TO THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TO THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NA							management of the Park Management (Cal		

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Timing Plan: PM Peak Hour

	•	Y	1	1	1	1			
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	γ			स	ß				
Volume (vph)	1	1	- 1	195	422	3			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Lane Width (ft)	12	12	12	10	10	12			
Grade (%)	0%			-1%	-2%	est comment of the state of the			
Link Speed (mph)	25			35	35				
Link Distance (ft)	378			763	712		sandi tamini sasa disentati	INTO TORSE HE PRODUCT SHEET HE AND HEAD OF	
Travel Time (s)	10.3			14.9	13.9				
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91			
Heavy Vehicles (%)	2%	2%	2%	1%	1%	2%			
Shared Lane Traffic (%)				ery sould all the College and Allen		100-100 CONTROL OF THE CONTROL OF TH			
Sign Control	Stop			Free	Free				
Intersection Summary		1000	signal del				45.00	(2. S. C.)	
Area Type:	Other								

Control Type: Unsignalized

ntersection Delay, s/veh	0			SOLUTION MARKET SOURCE				
Movement	EBL	EBR	NBL	NBT		SBT	SBR	
/ol, veh/h		1	1	195		422	3	
Conflicting Peds, #/hr	0	0	0	0		0	0	
Sign Control	Stop	Stop	Free	Free		Free	Free	
RT Channelized	-	None	CONTROL CONTROL CONTROL	None			None	
Storage Length	0			•			- 10 P	
/eh in Median Storage,#	0		-	0		0	e Santaka dan sempakan saka	
Grade, %	0		-	-1		-2	•	
eak Hour Factor	91	91	91	91	en estanolella municipalitanolella (il	91	91	
Heavy Vehicles, %	2	2	2	1 1		1	2	
/lvmt Flow	1	1	1	214		464	3	
Major/Minor	Minor2		Major1			Major2		
Conflicting Flow All	681	465	467	0			0	
Stage 1	465	processing and the second seco	en.			-	•	
Stage 2	216		-1					
ollow-up Headway	3.518	3.318	2.218				#.	
ot Capacity-1 Maneuver	416	597	1094				•	
Stage 1	632		-			*		
Stage 2	820					na ska 14		
ime blocked-Platoon, %							*	
Mov Capacity-1 Maneuver	416	597	1094					
Nov Capacity-2 Maneuver	416			Apptoreur (Apptor Section (Apptor Section (Apptor Section (Apptor Section (Apptor Section (Apptor Section (App		ang Kalang Kalang Ka		
Stage 1	632						_	
Stage 2	819						**	
	EB		ND			SB		and the second of the second o
pproach ICM Control Delay, s	12.4		NB 0			0		
ICM LOS	12. 1 B		U			Y.		
Minor Lane / Major Mvmt	ne en e	IL NBT	EBLn1	SBT	SBR			
	109		490	וטט	OUIX	Name of the last		
Capacity (veh/h)					•			
ICM Cantrol Dolay (c)	0.00 8,29		0.004 12.4					
HCM Control Delay (s)				edine Sid				
HCM Lane LOS		A A	B					
ICM 95th %tile Q(veh)	0.00)3	0.014					
lotes								

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APPENDIX E AUXILIARY TURN LANE WARRANT ANALYSES

Left-Turn Lane Warrant Analysis

Two-Lane Highways

Based on PennDOT Publication 46 (October 2010 Edition), Section 11.17,

PROJECT INFORMATION

Project Number:	ESII.A.0000
Intersection:	Darby-Paoli Road & Site Driveway
Movement:	Left Turns to Site Driveway
Analysis Period:	2020 Projected (Build) Conditions - AM Peak Hour
Analyst:	EMM

<u>INPUTS</u>	Volume	Truck %	Terrain Type	T	PCE Volume
Advancing Volume $(V_A) =$	339	3%	Rolling	1.045	354
Opposing Volume $(V_0) =$	193	2%	Rolling	1.03	199
Number of Left Turns =	1	2%	Rolling	1.03	1
Speed Limit =	- 35	Т	ype of Control:	Unsi	gnalized
Proportion of Left Turns =	0.3%		Cycle Length:	60	sec.

WARRANT CALCULATION

$t_{w} = 0.76$	sec	Limiting ad	vancing	volume =	2666
$t_A = 10.17$	sec				
$t_{\text{median}} = 6.78$	sec	Left turn l	ane warr	rants are for ρ	> than
$\lambda = 0.42$	veh/hr	0.0225	for 3	5 mph	
$\mu = 1047$	veh/hr		$\rho =$	0.0004 no	t satisfied

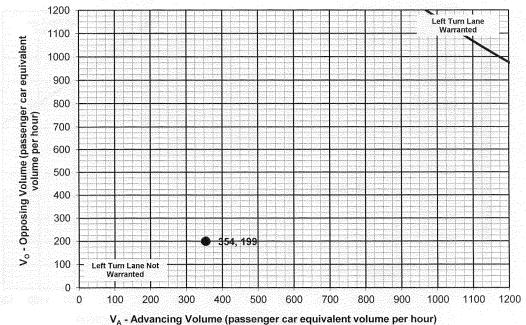
STORAGE LENGTH CALCULATION

Turn Demand Volume: Table 1 Condition: Avg # of veh/cycle:

Storage Length: feet

RESULTS

A left turn lane is not required.



Left-Turn Lane Warrant Analysis

Two-Lane Highways

Based on PennDOT Publication 46 (October 2010 Edition), Section 11.17,

PROJECT INFORMATION

Project Number:	ESII.A.0000
Intersection:	Darby-Paoli Road & Site Driveway
Movement:	Left Turns to Site Driveway
Analysis Period:	2020 Projected (Build) Conditions - PM Peak Hour
Analyst:	EMM

<u>INPUTS</u>	Volume	Truck %	Terrain Type	T	PCE Volume
Advancing Volume $(V_A) = [$	196	1%	Rolling	1.015	199
Opposing Volume $(V_0) =$	425	1%	Rolling	1.015	431
Number of Left Turns =	1	2%	Rolling	1.03	1
Speed Limit =	35	Type of Control:		Unsignalized	
Proportion of Left Turns =	0.5%		Cycle Length:	60	sec.

WARRANT CALCULATION

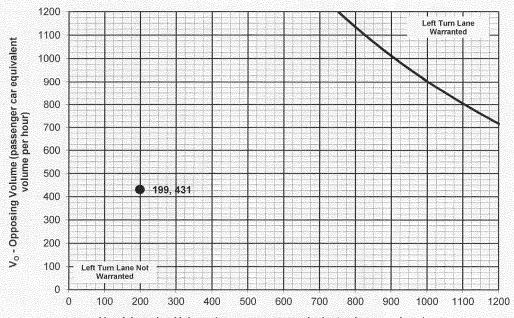
$t_{w} = 1.85$	sec	Limiting advancing volume = 1606
$t_A = 18.09$	sec	
$t_{\rm median} = 12.06$	sec	Left turn lane warrants are for $\rho \ge$ than
$\lambda = 0.31$	veh/hr	0.0225 for 35 mph
$\mu = 890$	veh/hr	$\rho = 0.0003$ not satisfied

STORAGE LENGTH CALCULATION

Turn Demand Volume:
Table 1 Condition:
Avg # of veh/cycle:
Storage Length:

RESULTS

A left turn lane is not required.

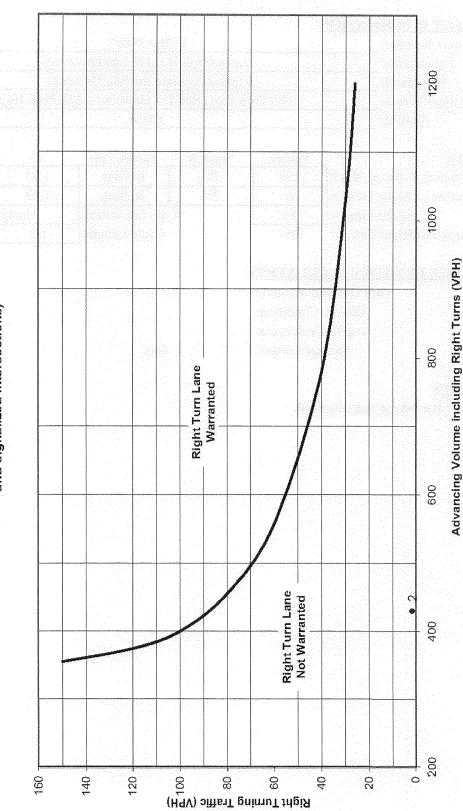


feet

Darby-Paoli Road & Site Driveway

Right-Turns into the Site Driveway

Figure 9. Warrant for right turn lanes on two-lane roadways (40 mph or lower speeds, unsignalized and signalized intersections)



1. 2020 Projected AM - 199 approach total, 0 right turns - NOT MET 2. 2020 Projected PM - 431 approach total, 3 right turns - NOT MET

Right-Turn Lane Warrant Analysis

Two-Lane Highways

Based on PennDOT Publication 46 (October 2010 Edition), Section 11.17

PROJECT INFORMATION

Project Number:	ESII.A.00001
Intersection:	Darby-Paoli Road & Site Driveway
Movement:	Right-Turns into the Site Driveway
Analysis Period:	2020 Projected (Build) Conditions - AM Peak Hour
Analyst:	EMM

<u>INPUTS</u>	Volume	Truck %	Terrain Type	T	PCE Volume
Advancing Volume $(V_A) = \int$	193	2%	Rolling	1.03	199
Number of Right Turns =	0	2%	Rolling	1.03	0
Speed Limit =	35	Type of Control:		Unsignalized	
Proportion of Right Turns =	0%		Cycle Length:	60	sec.

STORAGE LENGTH CALCULATION

Turn Demand Volume:

Table 1 Condition:

Avg # of veh/cycle:

Storage Length:

feet

RESULTS

A right-turn lane is not required.

Right-Turn Lane Warrant Analysis

Two-Lane Highways

Based on PennDOT Publication 46 (October 2010 Edition), Section 11.17

PROJECT INFORMATION

Project Number:	ESII.A.00001
Intersection:	Darby-Paoli Road & Site Driveway
Movement:	Right-Turns into the Site Driveway
Analysis Period:	2020 Projected (Build) Conditions - PM Peak Hour
Analyst:	EMM

INPUTS	Volume	Truck %	Terrain Type	T PCE Volume
Advancing Volume $(V_A) = $	425	1%	Rolling	1.015 431
Number of Right Turns =	3	2%	Rolling	1.03
Speed Limit =	35	1	ype of Control:	Unsignalized
Proportion of Right Turns =	1%		Cycle Length:	60 sec.

STORAGE LENGTH CALCULATION

Turn Demand Volume:

Table 1 Condition: Avg # of veh/cycle:

Storage Length:

feet

RESULTS

A right-turn lane is not required.